CHAPTER 8 Potential Unsewered Community Solutions

This Chapter presents further evaluation of the five preferred regionalization alternatives discussed in Chapter 3 and Chapter 4. This additional evaluation specifically identifies the infrastructure needs and total project costs to incorporate the wastewater generated from the twelve unsewered community clusters identified in Chapter 5 into the five alternatives. The Chapter is organized around the following sections:

- Unsewered Communities' Collection System Costs
- Alternative 1a
- Alternative 1c
- Alternative 2a
- Alternative 2b
- Alternative 3b
- Comparison of Alternatives

8.1 UNSEWERED COMMUNITIES' COLLECTION SYSTEM COSTS

This section presents the estimated capital and operating costs for new collection systems within each of the selected unsewered community clusters. The topics addressed are as follows:

- Cost Estimating Approach Overview
- Estimated Monte Rio/Villa Grande Area Collection System Costs
- Estimated Average Cost per Parcel for Remaining Community Clusters
- Estimated Costs for Community Cluster Collection Systems
- Community Cluster Collection System Operating Cost

8.1.1 Cost Estimating Approach Overview

The cost estimates presented in this section are common to all five alternatives, as follows:

- For the Monte Rio/Villa Grande area (Cluster 12), the common costs include the community
 collection system infrastructure, the community pump stations used to convey wastewater
 flows to the regional system, and the force main between the community collection system
 and the RRCSD WWTP. Under all of the alternatives, the Monte Rio/Villa Grande wastewater
 flows would be directed to the RRCSD WWTP site either for treatment in the WWTP or for
 export to another system along with the RRCSD flows.
- For the remaining community clusters, the common costs include the community collection system infrastructure and the community pump stations used to convey wastewater flows to the regional system. However, the force main costs for conveying wastewater from the community collection systems to a potential regional system can vary for each alternative. Therefore, these transmission main costs are included in the alternative-specific discussions presented later in this Chapter.

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As discussed in Chapter 1, the primary objective of this study is to assess the feasibility and costs for the wastewater infrastructure needed to support a regional wastewater system that provides service for existing sewered and unsewered communities within West County. However, to support this effort and provide a complete cost of each alternative, it is necessary to define the costs related to the removal of existing OWTS and construction of a new community collection system within the 12 community clusters.

The specific infrastructure required to provide wastewater collection within each community cluster will be unique to each area and this detailed level of analysis is beyond the scope of this current project. Therefore, readily available information was used to provide an estimate of these costs. For the Monte Rio/Vaille Grande area (Cluster 12) the estimated community collection system cost was developed directly from the information provided in the Monte Rio Study Report. For the remaining areas, cost information from Monte Rio Study Report and a recently bid septic-to-sewer conversion project located near Sacramento, California were used to define a range of average costs per connected parcel within each community cluster. To account for some of the unique features of each community, factors such as community size, flooding potential, parcel density, and access to major roadways were also considered in the development of the per parcel costs. Additional details regarding this approach are provided later in this section.

A detailed, community-specific evaluation will ultimately be needed to define the total collection system costs for each community. The site-specific average per parcel costs could vary significantly from the average per parcel costs identified herein. Moreover, site specific factors could result in the cost for an individual connection to a community system that ranges between 50 to 100 percent higher and 50 percent lower than the average per parcel costs defined for the community.

8.1.2 Estimated Monte Rio/Villa Grande Area Collection System Costs

For the Monte Rio/Villa Grande area (Cluster 12), the Monte Rio Study Report defines the community collection system cost, which also includes conveyance to the RRCSD WWTP, as \$51.5 million. Due to the potential for flooding in a significant portion of the community and this slope variation throughout the community, the collection system in the Monte Rio area is expected to be a pressurized system. The Monte Rio Study Report indicates that the estimated cost for this pressurized system would include the following elements:

- Traffic control;
- Existing septic tank decommissioning for each parcel;
- Grinder pump packages and electrical connection at each parcel;
- Lateral installations for each parcel;
- Various, 2-inch to 6-inch HDPE force mains that comprise the community collection system;
- Two community pump stations and a 6-inch transmission main to convey collected flow the RRCSD WWTP:
- Pipeline appurtenances, including air release valves and cleanouts;
- Two Caltrans bridge crossings;
- Nine local bridge crossings;
- Estimated 361 potential utility conflicts;
- Caltrans road restorations for both the transmission main and service force mains; and
- Local road restorations.

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Very little detail is provided regarding the development of the cost estimates that are included in the Monte Rio Study Report, and it is unclear whether the costs are representative of the construction cost or the total project costs¹. Additional engineering, environmental, legal and administrative costs may need to be added. The costs presented in the report also do not appear to include contractor-related costs such as mobilization/demobilization, contractor overhead and profit, and contractor general conditions. Finally, there does not appear to be an estimating contingency identified, which is appropriate for high-level cost estimate like the one presented.

A comparison of the cost information presented in the Monte Rio Study Report to the bid cost of a septic-to-sewer project in Sacramento indicates that the elements described in the paragraph above may not have been defined. Therefore, to ensure the costs presented herein are representative and consistent with the other cost estimates provide in this report, additional costs elements were included consistent with the approach used to develop the other costs described herein. Additional information about these various cost elements is provided the Basis of Cost Estimating TM included as Attachment A.

Table 8-1 provides the updated Project Cost. As noted previously, this cost includes all the infrastructure required to collect wastewater from 780 parcels within the Monte Rio/Villa Grande study area and deliver it to the RRCSD WWTP. Under this revised estimated, the average cost per parcel is \$139,600.

Table 8-1. Estimated Total Project Cost for Monte Rio/Villa Grande Community Collection System				
Cost Component		Cost, \$ million		
Community Collection System Cost Defined in N Study Report	Community Collection System Cost Defined in Mone Rio Study Report			
Contractor Costs				
Mobilization/Demobilization	15%	7.7		
Contractor's Overhead and Profit	5%	2.6		
Contractor's Overhead and Profit	10%	5.2		
	Subtotal	67.0		
Additional Project Elements				
Estimating Contingency	30%	20.1		
Engineer'	s Preliminary OPCC	87.1		
Engineering Design, Environmental Planning and Studies, Permitting, Construction Management, ESDC and Legal and Admin Costs, 25 percent of OPCC		21.8		
Engineer's Preliminary OPTCC		108.9		
ESDC = engineering services during construction				



¹ The Monte Rio Study Report initially labels the costs a construction costs, which implies they do not include non-construction project elements such as engineering design and permitting. However, in other areas of the report, the costs are presented as project costs, which implies these elements are included.



8.1.3 Estimated Average Cost Per Parcel for Remaining Community Clusters

For the remaining community clusters, three sources of cost information were used to define per parcel costs, as follows:

- The Monte Rio Study Report defines a cost for a Phase 1 pressurized collection system project that encompasses 550 parcels located in the flatter, higher-density areas of the Monte Rio/Villa Grande community. This cost was adjusted to remove the community pump stations and transmission main elements and to apply the contractor costs and estimating contingency factors described in Table 8-1. This results in a calculated Phase 1 construction cost pf \$53.4 million, or \$97,100 per parcel.
- The difference between the adjusted Phase 1 cost (\$53.4 million as described above) and the similarly adjusted total Monte Rio/Villa Grande construction cost (\$87.1 million as shown in Table 8-1) defines the cost for a potential Phase 2 pressurized collection system project that would encompass the 230 parcels located in the steeper-sloped, less dense areas of the Monte Rio/Villa Grande community. This estimated Phase 2 construction cost is \$27.5 million, or \$119,600 per parcel.
- The construction cost for a septic-to-sewer gravity system project located in a flat, relatively densely populated area near Sacramento, California of \$5.4 million for 51 parcels, or \$105,800 per parcel. This project was bid in January 2025 and is currently under construction.

These average per-parcel construction costs were further adjusted to account for some of the differences between the communities using the following approach:

- Flood risk: Clusters located in flood-prone areas were assigned a unit cost for a pressurized collection systems that were developed from the Monte Rio Study Report, while clusters that are not susceptible to flooding were assigned costs based on the Sacramento-area gravity system. Clusters with more than 30 percent of parcels in the floodplain² were assigned the flood risk cost category.
- Community size: The two project examples used to define the community collection system costs represent the high end (550 connections) and low end (51 parcels) of community sizes evaluated for this project. The smaller (51 connection) gravity system, which is in a flat area and has approximately 84 feet of lateral per connection, has a construction cost of \$105,800 per parcel. The larger (550 connection) Phase 1 pressure system from the Monte Rio Report, which is in a relatively flat area and has approximately 92 feet of lateral per connection, has a construction cost of \$97,100.



² Approximately 30 percent of the parcels in the Monte Rio/Villa Grande area are in the floodplain.

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While a pressurized system would be expected to have a lower per connection construction cost, there are likely some opportunities for economy of scale savings between the larger and smaller systems³. To adjust for these savings, the "power law scaling" has been applied as follows:

Cost Project A = Cost Project B x (Length of Pipeline in Project A / Length of Pipeline in Project B) $^{\alpha}$

where:

 α = 0.94 for gravity systems

 α = 0.95 for pressure systems

Community clusters with more than 350 parcels were assigned the larger-sized community cost and the community clusters with less than 200 parcels were assigned the smaller-sized community cost⁴.

• Slope conditions and parcel density: There is an approximate 19 percent difference between the estimated average Phase 1 and Phase 2 per parcel construction cost. This difference is assumed to represent the average difference in costs per parcel resulting from the higher construction difficulty for parcels that are in less dense areas and/or have steeper terrain⁵. Therefore, a 19 percent increase was applied to the average per parcel costs of the gravity system (which was in a flat sloped area) to define the cost of a gravity system in an area with higher slopes and/or less parcel density.

A summary of the average unit construction costs per parcel developed applying the methodology listed above is provided in Table 8-2.



³ Economy of scale savings are realized due to factors like increased efficiency, bulk purchasing discounts, and better utilization of specialized equipment and expertise.

⁴ No community clusters have a parcel count within the 200 and 350 parcel range.

⁵ A review of the available parcel density and parcel slope information confirms that higher sloped parcels are strongly correlated to lower density areas in the West County region.



Table 8-2. Average Unit Construction Costs for Septic-to-Sewer Conversions

System Type	Condition	Cost per Parcel, dollars	Basis			
Larger Communities (350 to 800 parcels) (a)						
	Flat slopes, no flood risk	99,700	Sized adjusted the average per parcel cost for the recently-bid, gravity system using the power law scaling equation			
Gravity System	High slopes, no flood risk	118,600	Adjusted the average per parcel cost for the flat-slope, no-flood-risk, larger-sized, gravity system defined above by the 19 percent factor included in Monte Rio Study Report			
Pressure System	Flat slopes, flood risk	97,100	Monte Rio Study Report Phase 1 cost, adjusted to remove Transmission Main and apply construction cost and estimating contingency factors described in Table 8-1 ^(b)			
	High slopes, flood risk	115,600	Size adjusted the smaller Monte Rio Study Report Phase 2 cost (which is based on 230 parcels) using the power law scaling equation.			
Smaller Communities (5	50 to 200 parcels) ^(a)					
	Flat slopes, no flood risk	105,800	Cost from recently bid project ^(c)			
Gravity System	High slopes, no flood risk	125,900	Adjusted the average per parcel cost for the flat-slope, no-flood-risk, smaller-sized, gravity system defined above by the 19 percent factor included in Monte Rio Study Report.			
Pressure System	Flat slopes, flood risk	100,500	Sized adjusted the larger Monte Rio Study Report Phase 1 cost shown above which is based on 330 parcels) using the power law scaling equation.			
	High slopes, flood risk	119,600	Monte Rio Study Report Phase 2 cost, adjusted to apply construction cost and estimating contingency factors described in Table 8-1 ^(d) .			

⁽a) No community clusters have a parcel count within the 200 and 350 parcel range.

⁽b) Phase 1 project cost with the transmission main removed and the construction cost and estimating contingency factors incorporated from Table 8-1 is estimated to be \$53.4 million.

⁽c) Project bid in January 2025 was \$5.4 million for 51 homes.

⁽d) Phase 2 project cost with the construction cost and estimating contingency factors incorporated from Table 8-1 is estimated to be \$27.5 million.



8.1.4 Estimated Community Collection System Costs

Table 8-3 presents the percent of parcels within each cluster that were shown to be impacted by baseline flooding (see Chapter 7 for details), as well as the percent of high-slope parcels within each cluster. As noted previously, costs for the Monte Rio/Villa Grande area (Cluster 12) were prepared separately and presented in the Monte Rio Study Report. Therefore, the flooding and slope percentage vales shown in Table 8-3 for Cluster 12 are only provided for comparison purposes.

	Table 8-3. Percent of Parcels Impacted by Baseline Flooding and with High Slopes				
ID	Name	Percent of Parcels Impacted by Flood Risk under Baseline Conditions	Percent of Parcels with Slopes >25 percent		
1	Guerneville South of River	11	32		
2	Guerneville North of River	29	16		
3	Northwood	45	3		
4	Hwy 116 East of Guerneville	6	19		
5	Summerhome Park Road	45	46		
6	Hacienda and Hollydale	39	15		
7	River Road North of Forestville	5	18		
8	Forestville	1	1		
9	Hwy 116 between Forestville and Graton	0	0		
10	Graton West	0	0		
11	Camp Meeker	0	11		
12	Monte Rio/Villa Grande	32	21		

Based on the information in Table 8-3, the following cost approach was applied related to flooding:

- Clusters 2, 3, 5, and 6 require pressurized systems, because they have similar percentage of parcels that are subject to flooding as the Monte Rio/Villa Grande system.
- The remaining systems are assumed to be predominantly gravity with smaller areas of pressured Septic Tank Effluent Pump (STEP) systems (if needed).

The percent of high slope parcels value in Table 8-3 was also used to develop a weighted average cost per parcel for each cluster.

The average per parcel construction costs and total estimated collection system construction costs are presented in Table 8-4 for all twelve clusters. The table also includes construction costs for the dedicated pump stations for each cluster to provide a total construction costs for the community collection system infrastructure common to all alternatives. Finally, a 25 percent factor was used to define the engineering, environmental, legal and administrative costs related to these projects, to define a total project cost.



Table 8-4. Unsewered Cluster Collection System Project Costs

ID	Name	Number of Parcels ^(a)	Estimated Average Collection System Construction Cost per Parcel, ^(b) dollars	Estimated Total Collection System Construction Cost,(c) \$ million	Estimated Pump Station Construction Cost, (d) \$ million	Estimated Total Construction Costs, \$ million	Estimated Total Project Costs ^(e) , \$million
1	Guerneville South of River	50	112,200	5.60	1.8	7.4	9.3
2	Guerneville North of River	60	103,600	6.20	1.7	7.9	9.9
3	Northwood	190	101,100	19.20	1.9	21.1	26.4
4	Hwy 116 East of Guerneville	110	109,600	12.10	1.8	14.0	17.5
5	Summerhome Park Road	100	109,300	10.90	1.8	12.8	16.0
6	Hacienda and Hollydale	440	99,900	44.00	1.9	45.9	57.4
7	River Road North of Forestville	450	103,100	46.40	1.9	48.3	60.4
8	Forestville	170	106,000	18.00	1.7	19.7	24.6
9	Hwy 116 between Forestville and Graton	140	105,800	14.80	1.7	16.5	20.6
10	Graton West	70	105,800	7.40	1.8	9.2	11.5
11	Camp Meeker	370	101,800	37.70	3.9 ^(f)	41.0	52.0
12	Monte Rio/Villa Grande	780			-	87.1 ^(g)	108.9
	Total	2,930		222.3	22.1	331.5	414.5

⁽a) Number of parcels from Table 5-5.

⁽b) The relevant unit costs from Table 8-2 were selected based on the cluster size and percent of parcels impacted by flooding from Table 8-3. Unit costs are also weighted based on the relevant unit costs from Table 8-2 and percent of parcels with relatively high slopes from Table 8-3.

⁽c) Total cost = number of parcels x average cost per parcel.

⁽d) The base construction costs for pump stations are proportional to the respective unsewered flows (see Table 5-7), with additional costs for Camp Meeker and Monte Rio/Villa Grande as noted.

⁽e) Cost shown includes an assumed 25 percent of the construction cost for Engineering Design, Environmental Planning and Studies, Permitting, Construction Management, ESDC and Legal and Admin Costs.

⁽f) Pump station cost for Camp Meeker also include \$2 million for upsizing the previously planned OCSD to Graton pump station and pipeline to accommodate Camp Meeker and Graton West flows.

⁽g) The Monte Rio/Villa Grande unit cost include two collection system community pump stations and the transmission main to RRCSD WWTP, which is common to all alternatives.

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As previously discussed, the costs shown in Table 8-4 are feasibility-level estimates, based on readily available information and are intended to provide the order of magnitude range of costs for potential future community collection systems. Development of more detailed site-specific costs for these community cluster collection systems is outside the scope of the current study.

8.1.5 Collection System Operating Costs

Similar to the project costs discussed above, the specific operating costs for providing reliable wastewater collection within each community cluster will be unique to each area and this detailed level of analysis is beyond the scope of this current effort. Therefore, average per- parcel O&M estimates have been developed to support this evaluation based on the information available in the Monte Rio Study Report. While the average costs presented likely reflect the order of magnitude cost for collection system O&M, actual costs could vary significantly what is defined herein depending on the system complexity and operational strategies. It is also important to note that these collection system O&M costs do not include the cost associated with O&M of the treatment systems that would receive wastewater from these community systems. These treatment-based O&M costs are presented later in this Chapter.

The Monte Rio Study Report listed a collection system O&M cost estimate of \$430,800 per year, which included costs associated with regular maintenance on low pressure collection mains, transmission force mains, individual grinder pump stations and main lift stations, equipment replacement costs, electricity and monitoring costs, and occasional emergency maintenance. No labor costs were included in this estimate. Instead, the analysis assumed inspections and maintenance would be the responsibility of the governing agency and applied the existing RRCSD monthly rates of \$190 for collection and treatment system O&M plus an estimated \$5 per month to cover the cost of electricity to run the grinder pump. This resulted in an estimated overall annual O&M cost of \$2,340 (\$195 per month) including treatment plant and collection system O&M.

Using the rates for current RRCSD customers is not representative of how rates would be established for the operations of collection systems in the unsewered communities. Moreover, some of the costs identified in the Monte Rio study would only apply to pressure systems. Therefore, the analysis presented herein relies on the \$430,800 per year estimated cost presented in the Monte Rio Study Report plus an estimated cost of two, full-time equivalent (FTE) employees at an assumed rate of \$200,000 a year per employee to define the annual collection system O&M cost for a 780-parcel sized pressure system. For gravity systems, the \$430,800 per year estimated cost presented in the Monte Rio Study Report was adjusted to remove the costs associated with maintenance of the grinder pumps and other associated equipment. However, the same FTE count was applied.

Table 8-5 provides the estimated average O&M costs for the pressurized and gravity systems on a per parcel basis. As shown, the operating costs per parcel is \$1,100 per year for a pressure system and \$700 per year for a gravity system. For a total unsewered parcel count of 2,930, total collection system operating costs for the unsewered communities is estimated to be approximately \$2.7 million.



Table 8-5. Collection System O&M Cost

	Cost, dollar		
O&M Cost Component	Pressurized System	Gravity System	
Scheduled Maintenance	24,000	24,000	
Line Cleaning	13,000	13,000	
Equipment Replacement	157,000	17,000	
Grinder Pump Electricity Cost	36,000	-	
Telemetry Monitoring	78,000	-	
Emergency Maintenance	43,000	43,000	
Transmission Pump Station O&M	73,000	73,000	
Transmission Pump Station Electricity Cost	6,800	6,800	
Labor	400,000	400,000	
Total Annual Costs	830,800	576,800	
Total Annual Cost per Parcel ^(a)	1,100	700	
Number of Parcels	1,570	1,360	
Total Annual O&M	\$1,730,000	\$950,000	
(a) Cost per parcel based on 780 parcels and total cost shown.			





8.2 ALTERNATIVE 1a

Alternative 1a involves providing treatment at two local facilities. Flows from FWD, GCSD, and OCSD would be treated at a combined FWD/GCSD WWTP and recycled water system that is sized to accommodate zero surface water discharge (i.e. in lieu of making treatment improvements to meet the nitrogen effluent limitations that have been prescribed for surface discharge). Flows from the RRCSD service area would continue to be treated at the existing RRCSD WWTP. This section presents a description of how the selected unsewered community clusters would be incorporated into this regional strategy, the associated basis of design for the facility improvements, a summary of the required facility improvements, site layout, project costs, O&M costs and total lifecycle costs.

8.2.1 Incorporation of Unsewered Clusters

Under Alternative 1a the selected unsewered community clusters would be incorporated into the regional facilities as follows:

- Clusters 1, 2, 3, and 12 would connect to the RRCSD WWTP for treatment at this site.
- Clusters 4⁶ through 8 would connect to the FWD WWTP for treatment at this site.
- Clusters 9 through 10 would connect to the GCSD WWTP for secondary treatment and then be discharged, along with the GCSD flows, to the FWD WWTP for tertiary treatment.
- Cluster 11 would initially be conveyed via the OCSD pump station and force main before traveling to the GCSD WWTP site with flows from Cluster 10.

8.2.2 Basis of Design

The projected flows and loads to RRCSD and FWD/GCSD WWTPs under Alternative 1a with unsewered community flows added in are presented in Table 8-6. These flows and loads assume equalization of RRCSD, GCSD, and FWD flows based on the information presented in Chapters 2 through 4. Flows and loads for the unsewered communities are taken from Chapter 5.

⁶ Although Cluster 4 is adjacent to a portion of the RRCSD service area, the RRCSD collection system is inadequate to receive an additional 48,000 gallons per day of peak flow. Therefore, so new conveyance would be needed to the RRCSD WWTP. Connecting to the force main that brings flow from Cluster 5 to the FWD WWTP is likely a more direct route. Additional evaluation of connection options for Cluster 4 is outside the scope of this effort.



Table 8-6. Alternative 1a Treatment Facilities Design Flows and Loads

Scenario	ADWF, mgd	Relevant Peak Flow Condition	Peak Design Flow, mgd	Maximum 30-Day BOD Load, lb/day
Design Flow for the RRCSD WWTP				
Projected RRCSD Flows and Load			4.2 ^(b)	2,620
Clusters 1-3 & 12	(a)	PDF	0.8	600
Total to RRCSD WWTP			5.0 ^(b)	3,220
Design Flow for the GCSD Screening System				
Projected GCSD/OCSD Flows and Load	(a)	PDF	1.5	(a)
Design Flows and Loads to the GCSD Secondary Trea	atment Ponds			
Projected GCSD/OCSD Flows and Load	0.15	MWF	1.1 ^(c)	680
Clusters 9-11	0.10	Peak Flow	0.38	300
Total to GCSD Ponds	0.25		1.5	980
Design Flows to the FWD Secondary Treatment Pon	ds			
Projected FWD Flows and Load	0.064	MMF	0.39 ^(c)	270
Clusters 4-8	0.20	Peak Flow	0.80	600
Total to FWD Ponds	0.26		1.2	870
Design Flows to the Combined Tertiary Filtration and Disinfection Facilities				
Combined FWS/GCSD/OCSD and Clusters 4-11	0.51		2.7	(a)

⁽a) Design condition is not relevant to the facility sizing.

⁽b) Equalized peak flow. The RRCSD Treatment Plant Master Plan indicates that Peak Day flows through the WWTP can be equalized to 5.0 mgd with use of existing Emergency Storage Pond. If 0.4 MG EQ basin is also available, flows can be equalized to 4.2 mgd. For this analysis, it is assumed flows can be equalized to 4.2 mgd. RRCSD Treatment Plant Master Plan indicates that un-equalized sustained peak flows are 5.2 mgd.

⁽c) Equalized peak flow. Chapter 4 discusses use of equalization storage to equalize peak flows to the treatment ponds.



8.2.3 Required Facility Improvements

This section addresses the required RRCSD treatment facility improvements, the required GSCD/FWD treatment facility improvements, and the required recycled water system improvements.

8.2.3.1 RRCSD Treatment Infrastructure Improvements

As discussed in Chapters 2 and 3, there are several condition and hydraulic capacity-related improvement projects that have been identified as necessary for continued long-term treatment at the RRCSD. In addition, based on the findings from the RRCSD Treatment Plant Master Plan summarized in Table 2-4, the RRCSD WWTP facilities should have adequate treatment capacity for up to 5.0 mgd peak flow⁷ and 3,220 lb/day BOD load with two notable exceptions:

- The tertiary filters are listed with a firm capacity of 4.0 mgd. It is assumed that a third,
 4.0 mgd filtration unit would be installed for this alternative to provide the necessary firm filtration capacity.
- Additional evaluation is needed of the RRCSD grit removal system performance to confirm expansion needs beyond the listed capacity of 4.1 mgd. No expansion of the grit removal is assumed for the current analysis.

Figure 8-1 shows the proposed layout of the RRCSD WWTP site.

8.2.3.2 FWD/GCSD Treatment Infrastructure Improvements

For the FWD/GSCD treatment systems, significant treatment improvements are needed to accommodate the projected flows and loads for those systems. Needed treatment improvements at the GCSD WWTP would remain unchanged under Alternative 1a with the addition of the Clusters 9, 10, and 11 flows. These improvements were detailed in Chapter 4 and include upgrades to the headworks and conversion of partial mix ponds to a complete mix system. Proposed treatment upgrades at the GCSD WWTP are the same as those outlined in Chapter 4 and already provided on Figure 4-3.

With the addition of flows and loads from Clusters 4 through 8, the existing partial mix ponds at the FWD WWTP would require upgrades to a complete mix system to provide sufficient treatment capacity. In addition, the following major treatment facilities would be required to accommodate tertiary treatment of the combined flows:

- Two, 1.5 mgd SAFs,
- Two, 2.7 mgd cloth disk filter units, and
- Four additional CCBs.

Figure 8-2 shows the proposed new infrastructure and treatment systems at the FWD WWTP, along with the existing infrastructure that will be repurposed.

⁷ Assuming the 0.4 mgd EQ basin will be available and RRCSD flows can be equalized to 4.2 mgd.







Prepared for:







Sonoma Water



8.2.3.3 Recycled Water Infrastructure Improvements at RRCSD

The RRCSD water balance was updated to account for the potential unsewered flows, with a finding that fifty-five additional acres of land application area would be required to accommodate approximately 93 AFY of additional recycled water from the RRCSD WWTP. However, no additional storage capacity would be needed.

It is assumed that RRCSD would be able to identify 55 acres of irrigated space on a combination of the 17-acre irrigation property or on the 394-acre forested property acquired in August 2024. RRCSD has not yet defined the acreage available for disposal on the 394-acre property. For the current analysis, expansion of irrigation/disposal facilities are assumed to cost \$25,000 per acre.

8.2.3.4 Recycled Water Infrastructure Improvements at FWD/GCSD

Zero discharge water balances for the combined FWD/GCSD system have been updated from those discussed in Chapter 3 to account for the addition of unsewered flows, as well as to address the findings presented in Chapter 6 of relatively low vineyard irrigation demands and additional demands from quarries near the FWD WWTP. Updated FWD/GCSD 100-year water balance results are presented in Table 8-7 for projected West County flows with and without the unsewered clusters.

Table 8-7. FWD/GCSD Water Balance Results for Alternative 1a						
Scenario	Storage Volume Required, acre-feet	Irrigated/Disposal Area Required, acres				
Existing Facilities						
FWD/GCSD	70 ^(a)	325 ^(b)				
FWD/GCSD Water Balance Results (1	FWD/GCSD Water Balance Results (100-Year Rainfall 75 in/yr)					
Projected	335	1,385				
Projected + Unsewered 690 3,365						
(a) Existing 7 acre-feet of storage at FWD assumed to be filled to support new treatment facilities.(b) Existing 20.5 acres of GCSD irrigation area assumed used for siting new recycled water storage.						

The updated water balance analysis indicates a need for the following:

- 3,040 acres of additional land application area⁸.
- 620 acre-feet of new recycled water storage, corresponding to a land acquisition need of about 52 acres⁹.

Figure 8-3 shows a potential footprint for the effluent storage ponds at the GCSD WWTP.

⁸ The parallel analysis in Chapter 3 included a conclusion that the existing irrigation FWD/GCSD reuse sites would be able to accommodate the combined recycled water flow generated (without unsewered flows). These updated results reveal that even with the additional quarry demands, more than 1,000 acres of additional land application area would be needed. The addition of the quarry demands did reduce the required additional recycled water storage without unsewered flows from 310 acre-feet to 265 acre-feet.

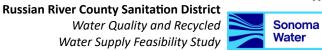
⁹ A detailed real estate assessment was not completed. Land purchase cost of \$100,000 per acre is assumed.



Prepared by:



Prepared for:





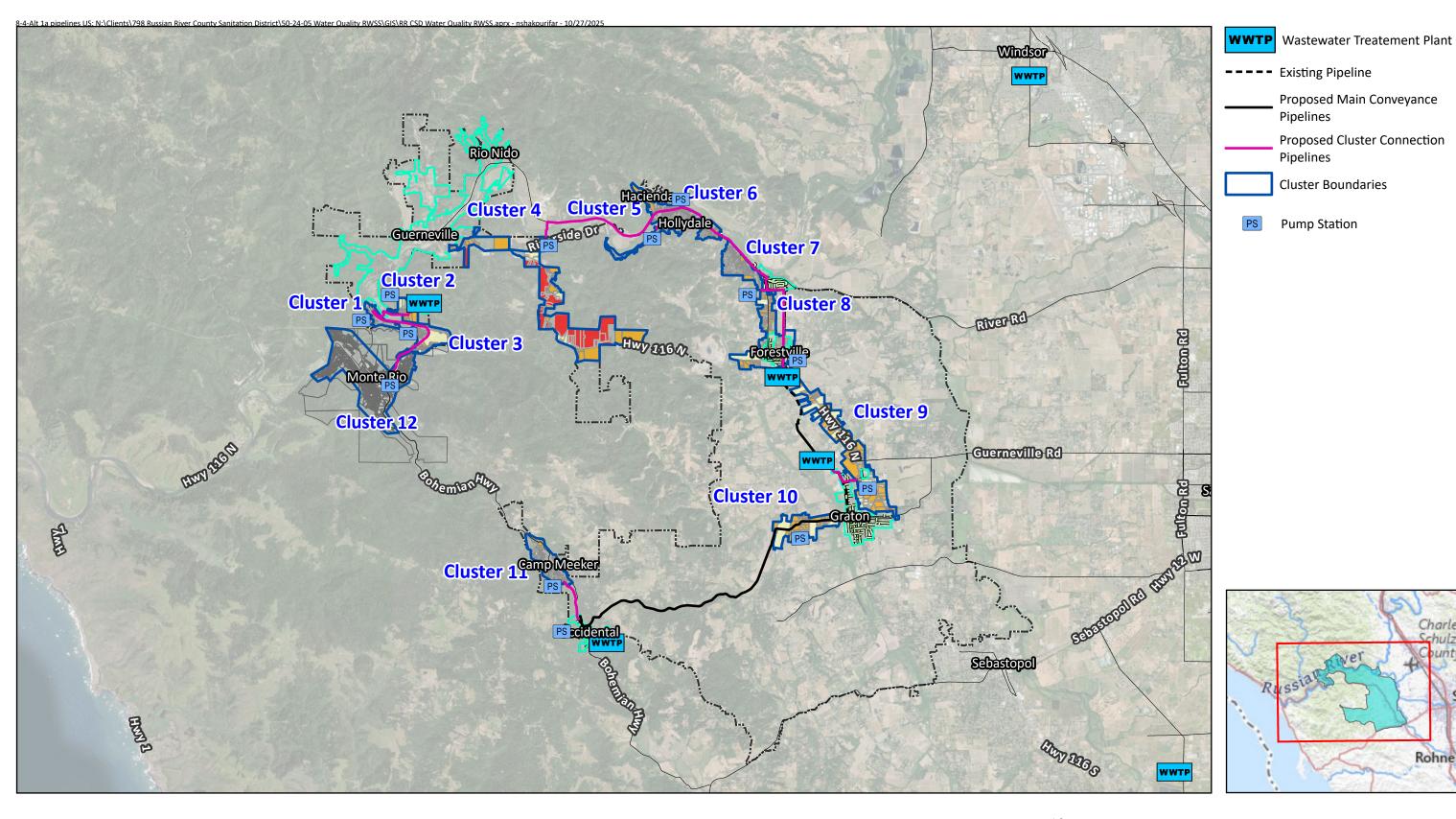
8.2.4 Conveyance Infrastructure

Relevant conveyance infrastructure includes collection system conveyance for the unsewered parcels, raw wastewater and secondary effluent conveyance to and from the GCSD WWTP and recycled water conveyance.

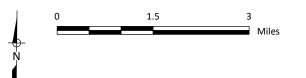
8.2.4.1 Collection System Infrastructure for Unsewered Community Clusters

Figure 8-4 presents a conceptual alignment of the forcemain conveyance facilities required to connect the community clusters lift stations to the appropriate regional WWTPs under Alternative 1a. The distances and diameters of these conveyance pipelines are summarized in Table 8-8.

Table 8-8. Sizing of Conveyance Infrastructure from Unsewered Areas for Alternative 1a					
Cluster(s)	Pipeline Distance, miles Pipeline Diameter, in				
Connection to RRCSD					
2	0.1	4			
3	1.2	6			
1, 3 & 12	1.7	8			
Connection to FWD/GSCD (a)					
4, 5, 9 & 11	4.0	4			
6 to 8	4.0	8			
Total Pipeline Length	11.0				
(a) Cluster 10 connects directly to the OCSD to GCSD pipeline.					







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Russian River County Sanitation District Water Quality and Recycled Water Supply Feasibility Study



Proposed Pipeline Alignments for Alternative 1a

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Figure 8-4

Charles

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8.2.4.2 Conveyance from OCSD to GCSD WWTP

The Occidental to Graton Pipeline Feasibility Study identified the need for a 4-inch pipeline to convey wastewater from the OCSD site to the GCSD collection system. With addition of Camp Meeker (Cluster 11) to the OCSD flows, peak flows would increase by 0.24 mgd, resulting in combined peak flows of 0.37 mgd. A larger conveyance pipeline and corresponding pump station than Sonoma Water is currently planning for will be required to accommodate the higher flows. The pipeline would need to be increased from 4- inch to a 6-inch diameter size. Cluster 10 also connects near the downstream end of the proposed alignment and is estimated to result in total peak flows of 0.41 mgd but would not require additional upsized pipeline. Capital costs for the upsized conveyance facilities are common to all alternatives and included in all alternatives to allow for comparison with the capital costs without unsewered communities added in.

8.2.4.3 Conveyance Between GCSD WWTP and FWD WWTP

To accommodate the flow of water between the GCSD and FWD facilities the following pipelines and pump stations are needed:

Secondary Effluent from GCSD to FWD:

- An existing pipeline would be rehabilitated and a new, parallel pipeline constructed to allow for transfer of up to 1.5 mgd of GCSD/OCSD secondary effluent from the GCSD WWTP to the FWD WWTP. The existing 1.7-mile long, 8-inch diameter DI pipeline between the FWD and GCSD WWTPs would be rehabilitated, and a new, parallel 0.8-mile, 6-inch pipeline would be constructed between the GCSD WWTP and the existing 6-inch PVC pipe connecting to the FWD WWTP.
- A new, 1.5 mgd effluent pump station to convey flow from the GCSD is also needed for secondary effluent transfer.¹⁰

Tertiary Effluent from FWD to GCSD Storage Ponds and Recycled Water Customers:

- A new, 1.7 mile 14-inch diameter pipeline would be constructed to allow for transfer of 2.7 mgd of tertiary effluent from the FWD WWTP to the storage ponds at the GCSD WWTP (and for distribution to recycled water customers).
- The existing 0.7 mgd effluent pump station at the FWD WWTP needs to be expanded to
 2.7 mgd to allow for transfer of flows from the FWD site to the ponds at the GCSD site.

• Tertiary Effluent from GCSD Storage Ponds to Recycled Water Customers:

 A new, 5.0 mgd¹¹ recycled water pump station at the GCSD site is needed to distribute recycled water from the storage ponds to recycled water customers.

The new pipelines would parallel the existing inter-tie pipelines between the FWD and GCSD WWTPs (see Figure 8-4).

¹⁰ GCSD has identified the need for a new effluent pump station. It is assumed that the facility would be sized to accommodate the proposed use. The new pump station is assumed to be adequately designed to accommodate the effluent flow detailed herein.

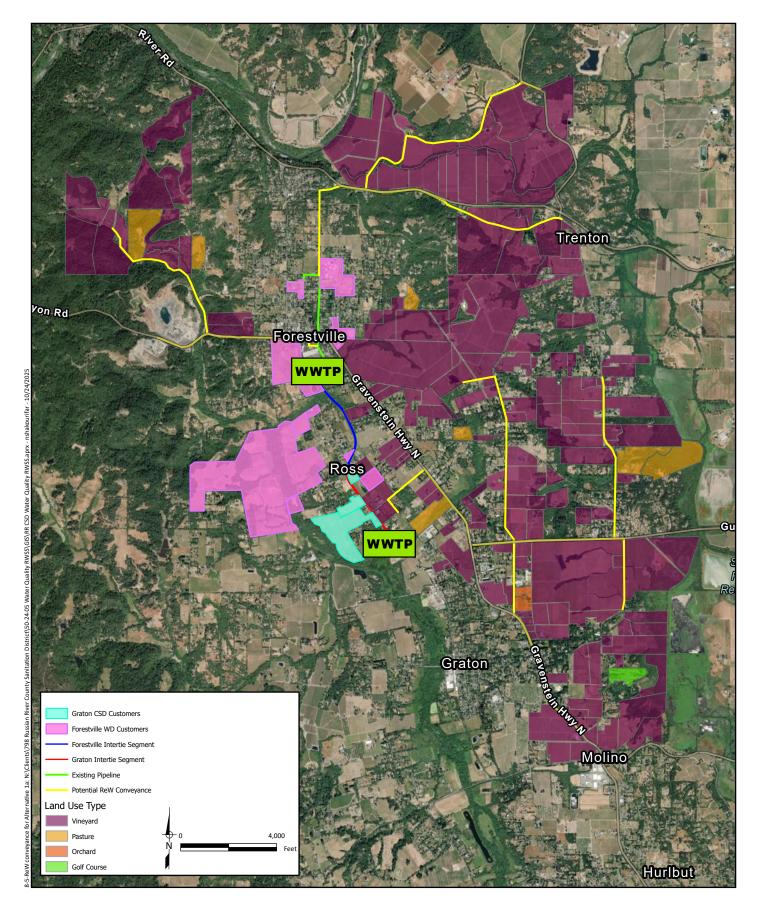
¹¹ The recycled water pump station has been sized based on the maximum month recycled water demand from the water balance, multiplied by an assumed peak day peaking factor of 1.5 to serve recycled customers.



8.2.4.4 Recycled Water Conveyance Infrastructure

A potential expanded recycled water service area and associated recycled water main service pipelines are presented on Figure 8-5. At a feasibility level, these conveyances are assumed to be 12-inch diameter. A total of 16 miles of conveyance pipeline is estimated, including pipelines to serve the nearby quarries. In addition, \$5,000 per acre is assumed to be needed to cover costs of additional piping and other required irrigation infrastructure on individual irrigated parcels.





Prepared by:



Prepared for:





8.2.5 Project Costs

The OPCC and OPTCC for Alternative 1a project elements are presented in Table 8-9, which also shows the estimated cost shares for RRCSD, FWD/GCSD (which also includes costs for processing flows from OCSD) and for unsewered communities. Additional details regarding these costs are provided in a Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendix G-1.

The following cost share approach was used:

- Costs of improvements needed to accommodate treatment/disposal of all anticipated flows and loads are split between existing users and new customers based on their respective portion of capacity provided.
- Costs of RRCSD condition-related improvements are split between existing users and new customers based on their respective average flows.
- New customers pay a wastewater system connection fee based on existing rate structures for use of existing facilities.
- Any improvements or costs associated with collection systems or raw wastewater conveyance (including existing systems) are only applied to users of those systems (proportional to peak flow).





Table 8-9. OPCC and OPTCC for Alternative 1a

	Table 8-9. OPCC and OPTCC for A	Iternativ	e 1a		
Facility			Cost, \$	million	
Component	Components	RRCSD	FWD/GSCD	Unsewered	Total ^(a)
Treatment		\$16.6	\$20.0	\$30.4	\$67.0
RRCSD	Condition-related improvements and hydraulic capacity improvements	15.5		8.1	23.6
	Tertiary filter expansion	1.1		0.6	1.7
EMD	New SAF/cloth disk filtration facility and disinfection expansion		11.7	10.2	21.9
FWD	Convert treatment ponds to complete mix system		3.8	5.7	9.5
	Headworks improvements		0.7	0.2	0.9
GCSD	Convert treatment ponds to complete mix system		3.8	5.6	9.4
Conveyance		\$0.0	\$11.4	\$24.6	\$36.0
	1.5 mgd pump station at the GCSD WWTP for secondary effluent transfer to FWD		1.9	0.6	2.5
New Pump Stations	5.0 mgd pump station at the GCSD WWTP for tertiary effluent delivery to distribution system		2.2	1.8	4.0
	2.7 mgd pump stations at the FWD WWTP for tertiary effluent transfer to GCSD	-1	1.8	1.5	3.3
Pipeline	 Pipeline Connections Between FWD/GCSD: 0.8 mile, 6-inch pipeline extension of existing HDPE pipe 1.7 mile, 14-inch pipeline Rehab existing 1.7 mile, 8-inch ductile iron pipeline 	1	5.5	4.0	9.5
	11 miles of 4–8-inch community conveyance lines			16.7	16.7
Recycled Water		\$0.9	\$43.3	\$63.3	\$107.5
Land Appli-	55 acres (RRCSD)	0.9		0.5	1.4
cation Area	3,040 acres (FWD/GCSD)		6.2	9.0	15.2
Chama	620 acre-feet (GCSD)		13.9	20.2	34.1
Storage	52 acres land purchase		2.1	3.1	5.2
Pipeline to Use Areas	16 miles of 12-inch conveyance pipeline		21.1	30.5	51.6
Unsewered Serv Dedicated Pump	vice Area Systems (Collection Systems and o Stations)	1		\$331.7	\$331.7
	Engineer's Preliminary OPCC	\$18	\$75	\$450	\$542
-	ign, Environmental Planning and Studies, struction Management, ESDC and Legal and Admin at of OPCC ^(b)	5	19	113	136
	Engineer's Preliminary OPTCC	\$23	\$94	\$563	\$678
Was	tewater Agency's Treatment Plant Connection Fees	- \$6	- \$19	\$25	
	OPTCC with Connection Fees Included	\$17	\$75	\$588	
ESDC = engineering	services during construction		·		



8.2.6 Operations and Maintenance Costs

This section provides a summary of the O&M costs, focused on the following elements:

- Power costs
- Labor costs
- Chemical costs
- Equipment repair and replacement costs

Following this summary of the additional O&M elements, the total additional O&M costs are presented along with the 20-year, present-worth lifecycle O&M cost. Additional details regarding these O&M costs are provided in the Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendices H-1 and H-2.

8.2.6.1 Power Costs

The annual power costs for Alternative 1a are summarized in Table 8-10. These costs account for the energy demands of new treatment systems and conveyance infrastructure, as well as projected savings resulting from operational changes at the GCSD WWTP. Similar to the concepts presented in Chapter 4, an upgrade to a complete mix aeration system at the GCSD and FWD WWTPs increases power demands (reflected in the treatment pond power cost), but eliminating all tertiary treatment at the GCSD WWTP and eliminating membrane filtration at the FWD WWTP results in power savings.

Table 8-10. Annual Power Costs for Alternative 1a				
Cost Element Annual Cost, dollars				
RRCSD Treatment ^(a)	34,200			
Treatment Pond at GCSD and FWD WWTPs	303,200			
GCSD and FWD WWTP Operation(b)	-173,400			
Cloth Disk Filtration at FWD WWTP	900			
SAF at FWD WWTP	12,800			
Conveyance Pumps	100,500			
Total	\$278,200			

⁽a) Assuming addition of 20 percent of RRCSD current treatment power costs corresponding to increased loads.

8.2.6.2 Labor Costs

No change in labor costs was assumed in the analysis presented in Chapter 4. With unsewered flows included, it is assumed two additional full-time employees would be required for FWD/GCSD WWTP recycled water system O&M. The cost for one full-time employee is assumed to be \$200,000, so a total labor cost of \$400,000 per year is assumed.

⁽b) Assuming elimination of 90 and 60 percent of GCSD and FWD current treatment power costs. A negative cost represents cost savings relative to existing costs.



8.2.6.3 Chemical Costs

Annual chemical costs for Alternative 1a are summarized in Table 8-11. The bases for these costs are provided in the footnotes to the table. The savings assumed at the GCSD WWTP are the same as those used in Chapter 4. Additional RRCSD treatment costs would be incurred to treat the unsewered flows, and additional SAF and chlorine chemical costs would be incurred for FWD/GCSD treatment proportional to the increased flows from the unsewered communities.

Table 8-11. Annual Chemical Costs for Alternative 1a				
Cost Element Annual Cost, dollars				
RRCSD Treatment	12,500 ^(a)			
SAF floc aid	136,500 ^(b)			
CCB: chlorine gas and sulfur dioxide gas	53,800 ^(c)			
Current GCSD WWTP Operation	-69,700 ^(d)			
Total	\$133,100			

- (a) Additional chemical usage at RRCSD WWTP based on current chemical usage, assuming usage increase proportional to average flow.
- (b) Based on chemical usage at the existing GCSD SAF facility.
- (c) Additional chlorine gas and sodium bisulfite required for the expanded CCB at the FWD WWTP are estimated using FWD current design criteria and the projected increase in annual average flow from GCSD.
- (d) Assuming elimination of 100 percent of GCSD WWTP chemical costs. A negative cost represents cost savings relative to existing costs.

8.2.6.4 Equipment Repair and Replacement Costs

A summary of the estimated major equipment repair and replacement costs for Alternative 1a is provided in Table 8-12. As done with Chapter 4, replacement costs for equipment with replacement frequencies of more than 20 years were excluded from this analysis (e.g. pumps, which are assumed to need replacement after 25 years). These costs are higher than those used for Alternative 1a in Chapter 4, accounting most significantly for additional piping and valve maintenance and replacements.



Table 8-12. Equipment Repair and Replacement Costs for Alternative 1a

	Cost, dollars		
Cost Element	Annual Basis	Cost Basis (if different)	Cost Basis Assumption
Filter Cloth Replacement	3,000	21,000	Every 7 years
Routine O&M ^(a)	15,500	-	Annual
Piping and Valve Maintenance and Replacement Cost	233,800	-	5 percent of mechanical and piping capital costs
Instrumentation Maintenance	4,000	83,000	5 percent of instrumentation and controls capital costs, Year 15
Pumps Rebuilds and Major Maintenance	5,100	53,000	30 percent of pump capital costs, Every 10 Years
10-Year Parts Replacement ^(b)	200	2,000	Every 10 Years
Major Parts Replacement ^(c)	4,000	84,000	Every 15 Years
Total Annual Costs	\$266,000	-	

⁽a) Includes cloth disk filtration routine lubrication of backwash pumps, drive motor and gear box, SAF parts replacement, pond cleaning and pond blower filter/belt/oil changes.

8.2.6.5 Total Additional Annual O&M Costs

Total additional annual O&M costs are provided in Table 8-13 based on the information presented in the previous sections. Also shown are estimated cost shares, using the following approach:

- For treatment and recycled water costs, costs are divided between RRCSD and FWD/GSCD based on the facility where the costs are incurred.
- For collection system costs, any new O&M is for the unsewered communities, so those are shown assigned solely to the unsewered communities.

⁽b) Includes cloth disk filtration main "V-Ring" seal replacement.

⁽c) Includes aeration equipment replacement.

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Table 8-13. Additional Annual O&M Costs for Alternative 1a						
Service Area	RRCSD, \$		FWD/GCSD, \$		Total Annual OpEx, \$	
Treatment/Recycled Water						
Power	34,000		244,000		278,000	
Labor	-		400,000		400,000	
Chemicals	12,000		121,000		133,000	
Major Parts Replacement ^(a)	-		13,200		13,200 ^(b)	
Routine O&M for Treatment ^(a)	-		218,000		218,000 ^(b)	
Total Treatment Annual OpEx	\$46,000		\$996,200		\$1,042,200	
Collection	RRCSD	Unsewered	FWD/GCSD	Unsewered	Total	
Routine O&M for Conveyance ^(a)	-	8,000	-	26,800	34,800 ^(b)	
Collection System O&M ^(c)	-	1,150,000	-	1,529,000	2,679,000	
Total Collection Annual OpEx	-	\$1,158,000	-	\$1,555,800	\$2,713,800	

⁽a) The equipment repair and replacement costs from Table 8-12 are shown where relevant under treatment or collection.

8.2.6.6 Total 20-Year Present Worth of O&M Costs

To calculate total lifecycle costs for the alternative, total costs over the project lifetime (20 years) are needed. The total 20-year, present-worth O&M costs for Alternative 1a are shown in Table 8-14. Additional details are provided in Appendices H-1 and H-2.

Table 8-14. Present Worth O&M Cost for Alternative 1a				
O&M Cost Component	Total 20-Year Cost, \$ million			
Power	5.8			
Labor	8.4			
Chemicals	2.8			
Equipment Repair and Replacement	6.4			
Unsewered Collection Systems	56.3			
Total 20-Year, Present-Worth O&M Costs	\$80			

⁽b) These cost items comprise the total annual equipment repair and replacement cost in Table 8-12.

⁽c) Based on estimated costs shown in Table 8-5



8.2.7 Total Lifecycle Costs

A total lifecycle cost for Alternative 1a is calculated as shown in Table 8-15 using the OPTCC from Table 8-9 and total 20-year O&M costs from Table 8-14.

Table 8-15. Lifecycle Cost for Alternative 1a				
Cost Component	Cost, \$ million			
Total Project Capital Cost (CapEx)	678			
Total Present Worth O&M Costs (OpEx)	80			
Total Lifecycle Cost	\$758			

8.2.8 Potential Annual Operating Costs per ESD

The additional operating costs from Table 8-13 were combined with current operating costs to define potential operation costs per ESD under Alternative 1a¹². The following methodology was used to develop these per ESD annual operating costs:

- For treatment:
 - Current treatment operating costs from recent West County annual budgets were taken and divided amongst all prospective future users proportional to the number of ESDs.
 - Additional operating costs were taken from Table 8-13 and were proportioned by the same ratios.
- For collection systems:
 - The recent West County annual budgets were retained specific to the RRCSD and FWD/GSCD and none of these costs were allocated to unsewered communities¹³.
 - Additional operating costs (Table 8-13) were assigned only to the unsewered communities.
- The combined total operating costs values were divided by the respective number of ESDs.

These total annual operating costs and operating costs per ESD values are presented in Table 8-16. These per parcel costs include estimated operating costs only and do not include any costs related to debt that may arise from the implementation of a regional project. Therefore, the costs should not be interpreted as the annual charges paid by a sanitation district customer.

¹² Overall rates will include annual operating costs in addition to any appropriate debt service.

¹³ The unsewered community cluster collection system operating costs could potentially be consolidated amongst the system users, which would likely significantly reduce the collection system operating costs. Additional evaluation of governance and management options is recommended to better define these costs.



Table 8-16. Total Estimated Annual O&M Costs for Alternative 1a

Category	RRCSD	RRCSD Unsewered	FWD/GCSD	FWD/GCSD Unsewered	Total Annual Operating costs
Current Annual O&M for Treatment	\$4.1 M ^(a)	\$1.4 M ^(a)	\$0.8 M ^(b)	\$0.8 M ^(b)	\$7.1 M
Additional Annual O&M for Treatment	\$0.03 M	\$0.01 M	\$0.5 M	\$0.5 M	\$1.0 M
Current Collection System O&M	\$3.3 M ^(a)	-	\$0.9 M	-	\$4.2 M
Additional Collection System O&M ^(c)	-	\$1.2 M	-	\$1.6 M	\$2.8 M
Total Annual Operating costs	\$7.4 M	\$2.6 M	\$2.1 M	\$2.9 M	\$15.1 M
Total Annual Operating costs per ESD	\$2,100	\$2,100	\$1,200	\$1,500	

- (a) Current annual O&M from RRCSD FY 24/25 budgets split amongst proposed future users (proportioned by ESDs).
- (b) Current annual O&M from FWD/GSCSD FY 24/25 budgets split amongst proposed future users (proportioned by ESDs).
- (c) Based on \$49 per month per parcel from Monte Rio Study Report plus an assumed 2 new FTEs for collection system management. Totaling \$89 per parcel.

8.3 ALTERNATIVE 1C

Alternative 1c involves treating all West County flows at new treatment facilities constructed at the existing FWD WWTP site that provides the level of treatment required to allow for seasonal discharge to the Russian River. The system would also rely on the expansion of the FWD/GCSD recycled water system to allow for beneficial reuse during the dry-season period. ¹⁴ This section presents a description of how the selected unsewered community clusters would be incorporated into this regional strategy, the associated basis of design for the facility improvements, a summary of the required facility improvements, site layout, project costs, O&M costs and total lifecycle costs.

8.3.1 Incorporation of Unsewered Clusters

Under Alternative 1c the selected unsewered community clusters would be incorporated into the regional facilities as follows:

- Clusters 1, 2, 3, and 12 would connect to a pump station at the RRCSD site. This station would convey the combined RRCSD/community cluster flows to the FWD WWTP.
- Clusters 4 through 8 would connect to the same force main between RRCSD and FWD, ultimately discharging to the FWD WWTP.
- A dedicated Cluster 9 lift station and force main would be constructed to convey flows from this area directly to the FWD WWTP.

¹⁴ Existing recycled water use from RRCSD on the Northwood Golf Course is assumed to cease.



 Flow from Cluster 11 would be conveyed to the OCSD pump station where it would be combined with OCSD flows and discharged to a new discharge pump station at the GCSD WWTP site. Cluster 10 would also discharge to the new force main between the OCSD pump station and GCSD pump station. This new GCSD pump station would convey the combined GCSD/OCD/community cluster flows to the FWD WWTP.

8.3.2 Basis of Design

The projected flows and loads to the FWD WWTPs under Alternative 1c with unsewered community flows added in are presented in Table 8-17. These flows and loads are based on the information presented in Chapters 2 and 5 for the West County agencies and unsewered communities, respectively.

Table 8-17. Alternative 1c Treatment Facilities Design Flows and Loads					
Scenario	ADWF, mgd	Relevant Peak Flow Condition	Peak Design Flow, mgd	Maximum 30-Day BOD Load, lb/day	
Design Flow for the RRCSD Pump Station					
Projected RRCSD Flows and Load	0.38	MWF	3.5 ^(a)	2,620	
Clusters 1-3 & 12	0.20	Peak Flow	0.80	600	
Total to RRCSD Pump Station	(b)		4.3	(b)	
Design Flow to the GCSD Pump Station					
Projected GCSD/OCSD Flows and Loads	0.15	MMF	0.64 ^(c)	680	
Clusters 10-11	0.10	Peak Flow	0.28	300	
Total to GCSD Pump Station	(b)		0.92	(b)	
Design Flows to the FWD WWTP					
Projected FWD Flows and Loads	0.064	MMF	0.25 ^(c)	270	
Clusters 4-9	0.20	Peak Flow	0.92	2,760	
Combined West County and Unsewered	1.1		6.4	5,100	

⁽a) Equalized peak flow. Chapter 4 discusses use of existing storage to equalize peak flows to the new treatment system

⁽b) Design condition is not relevant to the facility sizing.

⁽c) Equalized peak flow. Chapter 4 discusses use of existing ponds to equalize peak flows to the new treatment system.



8.3.3 Required Facility Improvements

This section addresses the required RRCSD treatment facility improvements, the required GSCD/FWD treatment facility improvements, and the required recycled water system improvements.

8.3.3.1 RRCSD Treatment Infrastructure Improvements

Under this alternative, the following improvements will be required at the RRCSD WWTP site:

- The existing storage ponds and aeration basins at the RRCSD would be modified to provide equalization of RRCSD service area flows. It is assumed that flows could be equalized to 3.5 mgd. Unsewered flows would not be equalized. Required infrastructure for this conversion includes:
 - Influent control structures to allow flexible flow direction within each facility (e.g. directing flow to the ponds or pump station, or combining flows)
 - Additional internal pipelines within the facilities

Proposed infrastructure improvements at the RRCSD WWTP to convert existing storage ponds to EQ ponds are the same as those outlined in Chapter 4 and were already provided on Figure 4-4.

8.3.3.2 FWD/GSCD Treatment Improvements

Under this alternative, the following improvements will be required at the GCSD WWTP site:

- The GCSD headworks will be upgraded with a new mechanical screening facility.
- The existing GCSD and FWD treatment ponds would be reconfigured so to provide EQ at each site. With this approach, the GCSD and FWD flows can likely be equalized to the maximum 30-day average values (0.9 mgd combined total) (Unsewered flows would not be equalized.) Required infrastructure for this conversion includes:
 - Influent control structures at the GCSD and FWD sites to allow flexible flow direction within each facility (e.g. directing flow to the ponds or pump station, or combining flows)
 - A drain pump for the FWD ponds
 - Additional internal pipelines within the facilities
- A new 6.75 mgd, three train MBR treatment system that provides biological nitrogen removal will be installed at the FWD WWTP, paired with a new 6.75 mgd, three channel UV disinfection to meet treatment requirements.
- Two, 7.5 mgd fine screening units (one duty and one standby) with 2-millimeter openings will need to be installed upstream of the MBR system, as required by membrane manufacturers to protect the membranes from damage.
- An aerobic solid digestion system with a capacity of handling 50,000 gallons per day of WAS
 will be constructed to treat WAS generated by the MBR process. A new solids handling
 building will house mechanical equipment, such as solids thickening and dewatering
 systems, blowers, and a cake storage room.

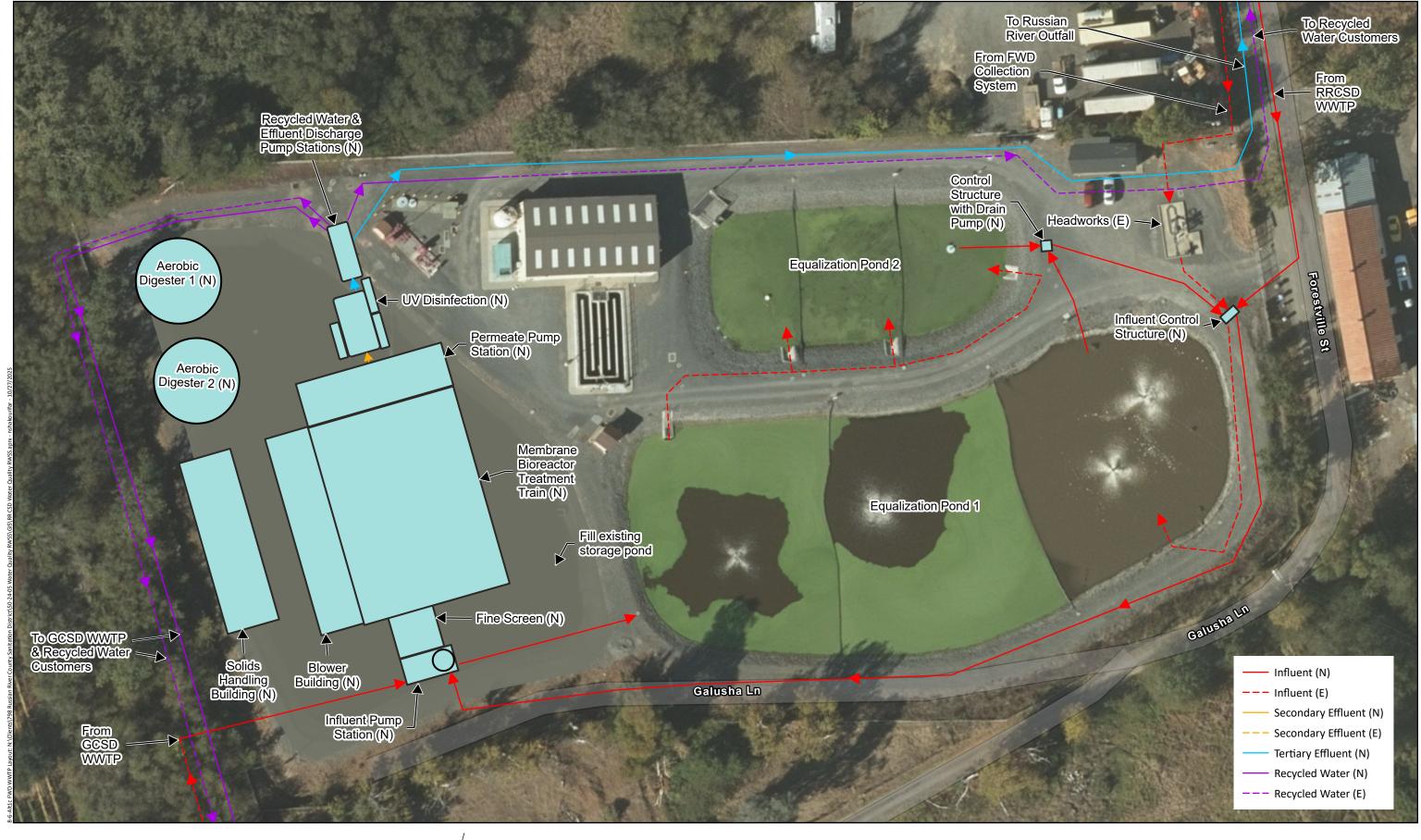
Chapter 8

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These improvements are the same as those presented in Chapter 4 for Alternative 1c, except capacities have been approximately doubled for account for the increased flows and loads from the unsewered communities. The site layout showing the proposed improvements at the FWD WWTP, along with the existing infrastructure that will be repurposed, is provided as Figure 8-6. Proposed infrastructure improvements at the GCSD WWTPs to convert existing treatment ponds to EQ ponds are the same as those outlined in Chapter 4 and were provided on Figure 4-5.









Sonoma Water



8.3.3.3 Recycled Water Infrastructure Improvements

As with Alternative 1a, water balances for the combined FWD/GCSD system with all West County flows have been updated from those discussed in Chapter 3 to account for the addition of unsewered flows, as well as to address the findings presented in Chapter 6 of relatively low vineyard irrigation demands and additional demands from quarries near the FWD WWTP. Updated FWD/GCSD 100-year water balance results are presented in Table 8-18 for projected West County flows with and without the unsewered clusters.

Table 8-18. FWD/GCSD Water Balance Results for Alternative 1c					
Scenario	Storage Volume Required, acre-feet	Irrigated/Disposal Area Required, acres			
Existing Facilities					
FWD/GCSD	70 ^(a)	345			
FWD/GCSD Water Balance Results (100-Year Rainfall 75 in/yr)					
Projected	44	345			
Projected + Unsewered	255	325 ^(b)			
(a) Existing 7 acre-feet of storage at FWD assumed to be filled to support new treatment facilities.(b) Existing 20.5 acres of GCSD irrigation area assumed used for siting new recycled water storage.					

The updated water balance analysis indicates the following:

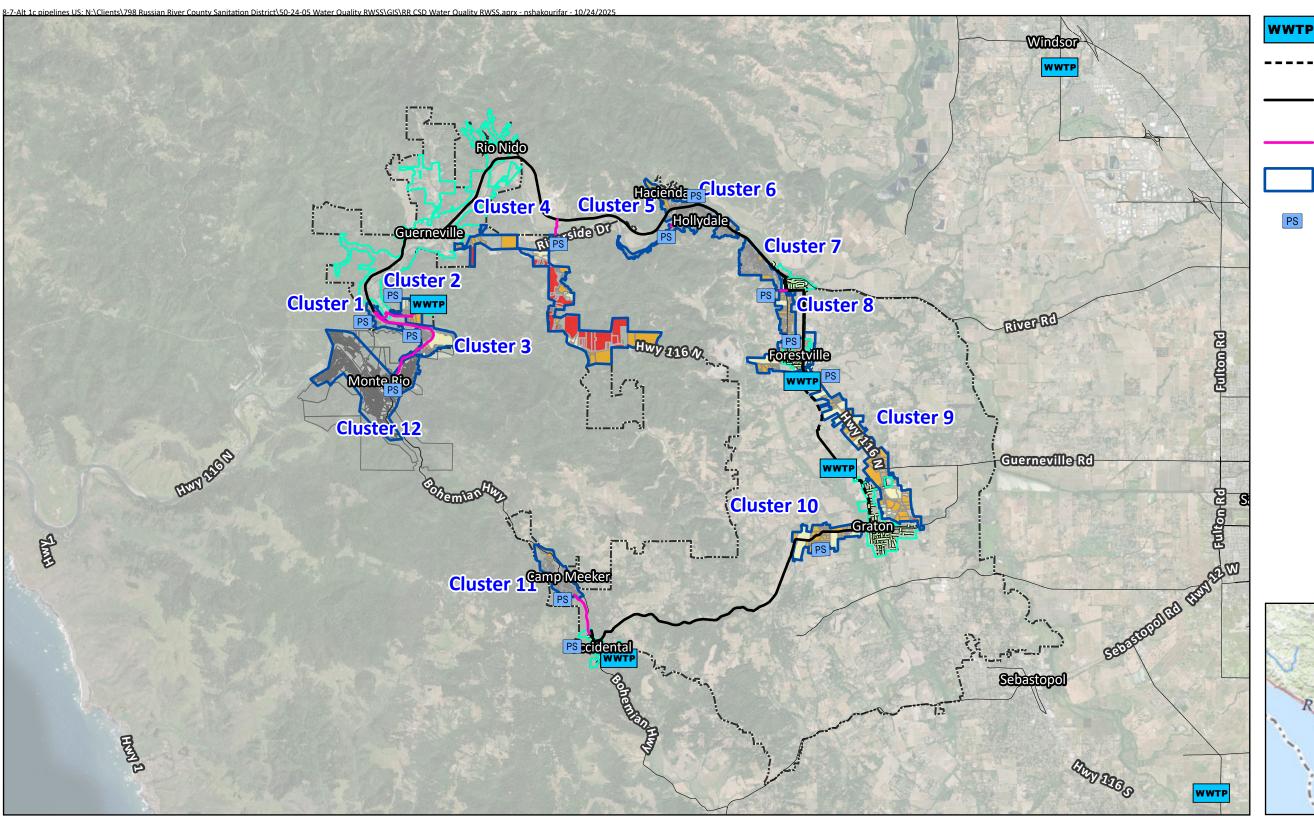
- No additional land application area required.
- 185 acre-feet of new recycled water storage needed, which could be sited on the available 20.5-acre GCSD land application site. A proposed location of the recycled water facilities at GCSD for effluent storage are the same as that presented in Chapter 4 on Figure 4-3.
- Extension of the existing recycled water distribution system to the quarries near Forestville would also be required.

8.3.4 Conveyance Infrastructure

Relevant conveyance infrastructure includes collection system conveyance for the unsewered parcels, raw wastewater conveyance to the GCSD and FWD WWTPs, treated effluent conveyance between the GCSD and FWD WWTPs and conveyance to the Russian River and recycled water users.

8.3.4.1 Collection System Infrastructure for Unsewered Community Clusters

A new 10- to 24-inch, 13-miles of pipeline between the RRCSD and FWD WWTP sites would be constructed. This pipeline would be constructed along River Road and have two river crossings. A conceptual alignment is shown on Figure 8-7.



WWTP Wastewater Treatement Plant

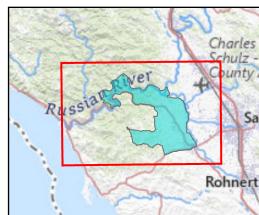
---- Existing Pipeline

Proposed Main Conveyance
Pipelines

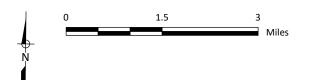
Proposed Cluster Connection
Pipelines

Cluster Boundaries

Pump Station







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Figure 8-7

Potential Unsewered Community Solutions



In addition, each cluster would have a new pipeline connection as summarized in Table 8-19.

Table 8-19. Sizing of Conveyance Infrastructure from Unsewered Areas for Alternative 1c					
Cluster(s) ^(a) Pipeline Distance, miles Pipeline Diameter, inches					
2, 4, 5, 9 & 11	1.3	4			
7	1.3	6			
3 & 12	0.8	8			
Total Pipeline Length	3.4				
(a) Clusters 1, 6 and 8 connect directly to the RF	RCSD to FWD pipeline, and Cluster 10 conne	cts directly to the OCSD to GCSD pipeline.			

8.3.4.1 Raw Wastewater Conveyance

To accommodate conveyance of raw wastewater between the RRCSD and GCSD facilities to the FWD WWTP, the following pipelines and pump stations are needed:

- A new 4.3 mgd pump station at the RRCSD WWTP and 13 miles of 10 to 24-inch pipeline to convey raw wastewater to the FWD WWTP site.
- The existing 1.7-mile long, 8-inch diameter DI pipeline between the FWD and GCSD WWTPs would be rehabilitated to allow for transfer of 0.64 mgd of equalized, raw OCSD/GCSD wastewater to the FWD site.
- A new 0.64 mgd effluent pump station for GCSD is needed for raw wastewater transfer to the FWD WWTP site.

8.3.4.2 Treated Effluent and Recycled Water Conveyance

The following major conveyances will be required for discharge or transfer of effluent and recycled water:

- A new 4.1-mile long, 30-inch diameter outfall from the FWD WWTP to the Russian River (A proposed alignment for the outfall was provided as Figure 3-6 in Chapter 3).
- New pipelines and pump stations will be constructed between the FWD and GCSD sites to convey flows to the treatment, storage, and recycled water facilities, as follows:
 - Treated Water from FWD to GCSD: A new, 1.7-mile, 16-inch pipeline between the GCSD WWTP and the FWD WWTP would be constructed to allow for transfer up to 6.4 mgd of tertiary effluent from the FWD WWTP to the GCSD storage ponds and/or the existing discharge location at the GCSD WWTP. A new 6.4 mgd effluent pump station at the FWD site needs to be constructed to allow for discharge to surface waters and/or transfer to the GCSD ponds.



- Recycled Water from GCSD to Existing Recycled Water Customers: A new 0.8-mile, 6-inch pipeline between the GCSD WWTP and the existing 6-inch PVC pipe connecting to the FWD WWTP would be constructed to allow for transfer of 0.6 mgd of tertiary effluent from the storage ponds at the GCSD WWTP to existing recycled water customers. A new, 0.8 mgd¹⁵ recycled water pump station at the GCSD site is needed to distribute recycled water from the storage ponds to recycled water customers.
- Recycled Water to Quarries: New 1.4 miles of 12-inch conveyance pipeline would be needed from the FWD WWTP to the nearby quarries, similar to what is shown on Figure 8-5.

8.3.5 Project Costs

The OPCC and OPTCC for Alternative 1c project elements are presented in Table 8-20, which also shows the estimated cost shares for RRCSD, FWD/GCSD and for unsewered communities. Additional details regarding these costs are provided in a Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendix G-2.

The following cost share approach was used:

- Costs of improvements needed to accommodate treatment/disposal of all anticipated flows and loads are split between existing users and new customers based on their respective portion of capacity provided.
- Cost of EQ improvements are applied to each community (i.e. RRCSD, GCSD and FWD).
- No connection fee is assumed because basically all new treatment infrastructure is being constructed.
- Any improvements or costs associated with collection systems or raw wastewater conveyance (including existing systems) are only applied to users of those systems (proportional to peak flow).

¹⁵ The recycled water pump station has been sized based on the maximum month recycled water demand from the water balance, multiplied by an assumed peak day peaking factor of 1.5 to serve recycled customers.



Table 8-20. OPCC and OPTCC for Alternative 1c

Table 8-20. OPCC and OPTCC for Alternative 1c							
Facility		Cost, \$ million					
Component	Components	RRCSD	FWD/GCSD	Unsewered	Total ^(a)		
Treatment		\$43.7	\$24.2	\$32.3	\$100.1		
RRCSD	Convert storage ponds to EQ facility	0.3			0.3		
Convert treatment ponds to EQ ponds			3.8		3.8		
FWD	Construct new BNR/MBR facility + UV disinfection + Solids Processing	43.4	19.4	32.2	94.9		
CCCD	Headworks improvements		0.8	0.1	0.9		
GCSD	Convert treatment ponds to EQ facility		0.2		0.2		
Conveyance		\$102.1	\$13.5	\$42.5	\$158.1		
	4.3 mgd pump station at RRCSD discharge to FWD site	3.1		0.8	3.9		
New Pump Stations	Two pump stations at GCSD for raw wastewater transfer to FWD site (0.6 mgd) and discharge of recycled water to customers (0.8 mgd)		3.5	1.5	5.0		
	6.4 mgd pump station at FWD for Russian River discharge and for recycled water to storage/reuse	2.4	0.6	1.4	4.4		
	13-mile of 10 to 24-inch conveyance pipeline between RRCSD and FWD	70.6		18.5	89.1		
Pipeline	 Pipelines connections between FWD/GCSD: 1.7 mile, 16-inch pipeline for tertiary effluent transfer between GCSD and FWD 0.8 mile, 6-inch pipeline extension of existing HDPE pipe Rehab existing 1.7 mile, 8-inch pipeline between GCSD and FWD 	4.7	3.9	4.4	13.0		
	4.1-mile, 30-inch Conveyance and Outfall to Russian River	21.3	5.5	12.2	39.0		
	3.4 miles of 4–8-inch community conveyance lines			3.7	3.7		
Recycled Wate	r	\$5.5	\$3.1	\$7.4	\$16.0		
Storage	185 acre-feet (GCSD)	5.0	2.8	6.7	14.5		
Pipeline to Use Areas	0.5 miles of 12-inch conveyance pipeline to quarries	0.5	0.3	0.7	1.5		
Unsewered Ser Dedicated Pum	rvice Area Systems (Collection Systems and up Stations)			\$331.7	\$331.7		
	Engineer's Preliminary OPCC	\$151	\$41	\$414	\$606		
Permitting, Cor	sign, Environmental Planning and Studies, astruction Management, Engineering Services ction and Legal and Admin Costs, 25 percent of to all)	41	11	105	157		
	Engineer's Preliminary OPTCC	\$192	\$52	\$519	\$763		
(a) Project Phase	-Level OPCC contingency of 30 percent applied to all element	S.	•	•			



8.3.6 Operations and Maintenance Costs

This section provides a summary of the O&M costs for Alternative 1c, focused on the following elements:

- Power costs
- Labor costs
- Chemical costs
- Equipment repair and replacement costs

Following this summary of the additional O&M elements, the total additional O&M costs are presented along with the 20-year, present-worth lifecycle O&M cost. Additional details regarding these O&M costs are provided in the Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendices H-3 and H-4.

8.3.6.1 Power Costs

The annual power costs for Alternative 1c are summarized in Table 8-21. These costs account for power for the new tertiary treatment facility, solids handling facility and conveyance infrastructure, as well as anticipated savings from operational changes at the RRCSD, FWD, and GCSD WWTPs. These costs are higher than those used in Chapter 4 for Alternative 1c because of increased treatment requirements with the unsewered flows.

Table 8-21. Annual Power Costs for Alternative 1c				
Cost Element Cost, dollars				
Fine Screen	2,000			
MBR Treatment System	242,200			
Aerobic Digestion	86,600			
UV	86,700			
Conveyance Pumps	287,400			
RRCSD, FWD and GCSD WWTPs Operation ^(a)	-576,600			
Total	\$128,300			
(a) Assuming elimination of 95, 90 and 60 percent, respectively, of RRCSD, GCSD and FWD WWTP current treatment power costs. A negative cost represents cost savings relative to existing costs.				

8.3.6.2 Labor Costs

The following labor cost reductions are assumed for this alternative, same as in Chapter 4:

- 100 percent increase in labor and administrative costs at the FWD WWTP
- 75 percent reduction in labor costs at the RRCSD WWTP
- 90 percent reduction in labor and a 50 percent reduction in administrative costs at the GCSD WWTP

Based on these assumptions, the estimated annual labor cost savings is \$2.9 million.

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In addition, the expanded recycled water storage needs are assumed to require 2 full-time employees, at a total cost of \$400,000 per year.

8.3.6.3 Chemical Costs

Annual chemical costs for Alternative 1c are summarized in Table 8-22. The basis for these costs are provided in the footnotes to the table. The savings assumed at the RRCSD and GCSD WWTP are the same as those used in Chapter 4. Additional MBR and digestion chemical costs would be incurred to treat the unsewered flows, proportional to the increased flows.

Table 8-22. Annual Chemical Costs for Alternative 1c				
Cost Element Cost, dollars				
MBR Treatment System	24,200 ^(a)			
Aerobic Digestion	32,800 ^(b)			
RRCSD and GCSD WWTPs Operation	-104,700 ^(c)			
Total	-\$47,700			

- (a) Based on chemical doses provided by manufacturer for system cleaning after organic fouling events.
- (b) Based on estimated polymer usage for dewatering and thickener units.
- (c) Assuming elimination of 100 percent of RRCSD and GCSD WWTPs chemical costs. A negative cost represents cost savings relative to existing costs.

8.3.6.4 Equipment Repair and Replacement Costs

A summary of the estimated major equipment repair and replacement costs for Alternative 1c is provided in Table 8-23. Replacement costs for equipment with replacement frequencies of more than 20 years were excluded from this analysis (e.g. pumps). These costs are higher than those used for Alternative 1c in Chapter 4, with several categories estimated to require significantly higher costs.



Table 8-23. Equipment Repair and Replacement Costs for Alternative 1c

	Cost, dollars		
Cost Element	Annual Basis	Cost Basis (if different)	Cost Basis Assumption
UV Equipment Replacement	36,000	-	Annual
Piping and Valve Maintenance and Replacement Cost	525,000	-	5 percent of mechanical and piping cost
MBR Membrane Cassette	69,000	-	Every 7 years
Instrumentation Maintenance	12,300	259,000	5 percent of instrumentation and controls cost, Year 15
Pumps Rebuild and Major Maintenance	21,100	221,800	30 percent of pump cost, Every 10 Years
15-Year MBR Replacements	18,300	384,000	Every 15 years
20-Year MBR Replacement	30,000	630,000	Every 20 years
20-Year Major Equipment Replacement (fine screens)	60,400	1,269,000	Every 20 years
RRCSD and GCSD Operations ^(a)	-948,600	-	Annual
Total Annual Costs	-\$176,500		

a) Assumes a 90 percent cost reduction for parts replacement, permitting, and testing/analysis at the RRCSD WWTP; a 50 percent reduction in SCADA-related costs at RRCSD; and an 80 percent reduction in equipment maintenance costs at the existing GCSD WWTP.

8.3.6.5 Total Additional Annual O&M Costs

Total additional annual O&M costs are provided in Table 8-24 based on the information presented in the previous sections. Also shown are estimated cost shares, using the following approach:

- For treatment and recycled water costs, costs are divided between RRCSD and FWD/GSCD based on the facility where the costs are incurred.
- Routine O&M for conveyance refers to the major conveyance pipelines, and those costs are divided proportional to peak flows.
- For collection system costs, any new O&M is for the unsewered communities, so those are shown assigned solely to the unsewered communities.

Table 8-24. Additional Annual O&M Costs for Alternative 1c							
Service Area	RRCSD, \$	FWD/GCSD, \$ Total Annual Op					
Treatment/Recycled Water	Treatment/Recycled Water						
Power	-218,000	343,000	125,000				
Labor	-2,964,000	-16,000	-2,980,000				
Chemicals	-35,000	-13,000	-48,000				
Major Parts Replacement	5,000	137,000	142,000 ^(a)				
Routine O&M for Treatment	-847,000	504,000	-343,000 ^(a)				
Total Treatment Annual OpEx	- \$4,059,000	\$955,000	-\$3,104,000				
Collection	RRCSD	FWD/GSCD Unsewered	Total				

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Total Collection Annual OpEx	\$126,000	\$8,000	\$2,757,000	\$2,891,000
Collection System O&M	-	ı	2,679,000	2,679,000
Routine O&M for Conveyance	126,000	8,000	78,000	212,000 ^(a)

⁽a) These cost items comprise the total annual equipment repair and replacement cost in Table 8-23 with the addition of \$188,000 a year for solids handling.

8.3.6.6 Total 20-Year Present Worth of O&M Costs

The total 20-year, present-worth O&M costs for Alternative 1c are shown in Table 8-25. Additional details are provided in Appendices H-3 and H-4.

Table 8-25. Present Worth O&M Cost for Alternative 1c				
O&M Cost Component Total 20-Year Cost, \$ million				
Power	2.6			
Labor	-62.6			
Chemicals	-1.0			
Equipment Repair and Replacement	-3.7			
Dewatered Solids Hauling	4.0			
Unsewered Collection Systems	56.3			
Total 20-Year, Present-Worth O&M Costs	-\$4.4			

8.3.7 Total Lifecycle Costs

A total lifecycle cost for Alternative 1c is calculated as shown in Table 8-26 using the OPTCC from Table 8-20 and total 20-year O&M costs from Table 8-25.

Table 8-26. Lifecycle Cost for Alternative 1c				
Cost Component Cost, \$ million				
Total Project Capital Cost (CapEx)	763			
Total Present Worth O&M Costs (OpEx)	-4.4			
Total Lifecycle Cost	\$759			

8.3.8 Potential Annual Operating Costs per ESD

As with Alternative 1a, the additional operating costs (Table 8-24) can be combined with current operating costs to define potential future annual operating costs per ESD. The same methodology used for Alternative 1a was applied for this alternative and are shown in Table 8-27. These per parcel costs include estimated operating costs only and do not include any costs related to debt that may arise from the implementation of a regional project. Therefore, the costs should not be interpreted as the annual charges paid by a sanitation district customer.

⁽b) Based on estimated costs shown in Table 8-5

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Table 8-27. Total Estimated Annual O&M Costs for Alternative 1c

Category	RRCSD	FWD/GSCD	Unsewered	Total Annual OpEx
Current Annual O&M for Treatment	\$5.5 M ^(a) + \$0.7 M ^(b)	\$0.3 M ^(b)	\$0.6 M ^(b)	\$7.1 M
Additional Annual O&M for Treatment	- \$4.1 M ^(c) + \$0.4 M ^(d)	\$0.2 M ^(d)	\$0.4 M ^(d)	- \$3.1 M
Current Collection System O&M	\$3.3 M ^(e)	\$0.9 M ^(e)	1	\$4.2 M
Additional Collection System O&M	\$0.1 M	-	\$2.8 M	\$2.9 M
Total Annual OpEx	\$5.9 M	\$1.4 M	\$3.8 M	\$11.1 M
Total Annual OpEx per ESD	\$1,600	\$800	\$1,200	

- (a) Current annual O&M from RRCSD FY 24/25 budget.
- (b) Current FWD/GSCD annual O&M costs split amongst proposed future users (proportioned by ESDs).
- (c) Savings from ceasing operations of RRCSD WWTP.
- (d) Additional cost for operating new FWD system (above existing cost). Total cost to operate new FWD system is: \$2.4 million.
- (e) Current annual O&M from FY 24/25 budgets.

8.4 ALTERNATIVE 2A

Alternative 2a includes conveying untreated West County flows to the Windsor WWTP for treatment. This section presents a description of how the selected unsewered community clusters would be incorporated into this regional strategy, the associated basis of design for the facility improvements, a summary of the required facility improvements, site layout, project costs, O&M costs and total lifecycle costs.

8.4.1 Incorporation of Unsewered Clusters

Under Alternative 1c the selected unsewered community clusters would be incorporated into the regional facilities as follows:

- Clusters 1, 2, 3, and 12 would be directed to a pump station at the existing RRCSD WWTP site that also conveys the RRCSD flows to the Windsor WWTP.
- Clusters 4 through 7 would connect directly to the force main between the RRCSD WWTP site and the Windsor WWTP.
- Cluster 11 would initially be conveyed to the OCSD pump station, combined with OCSD flows, and directed to a new pump station at the GCSD WWTP site that also conveys GCSD flows to the FWD WWTP.
- Cluster 10 would connect directly to the force main between OCSD and GCSD.
- Clusters 8 and 9 would be directed to a new pump station at the FWD WWTP site that also conveys FWD/GCSD/OCSD/Cluster 10-11 flows to the Windsor WWTP.



8.4.2 Basis of Design

The flows and loads of interest for design of the new treatment facilities under Alternative 2a are the same as for Alternative 1c with the difference being ultimate conveyance to and treatment at the Windsor WWTP instead of the FWD WWTP. These flows and loads of interest were presented in Table 8-17¹⁶.

8.4.3 Required Facility Improvements

This section addresses the required improvements to convert provide EQ at the RRCSD, GSCD, and FWD treatment facilities along with the estimated connection fee to cover costs of expanded treatment at the Windsor WWTP. These improvements are as follows:

- The existing storage ponds and aeration basins at the RRCSD would be modified to provide equalization of RRCSD service area flows¹⁷. A new influent control structure and internal pipelines would also be needed.
- The GCSD headworks will be upgraded with a new mechanical screening facility.
- The existing GCSD and FWD treatment ponds would be reconfigured so to provide EQ at each site¹⁸. Required infrastructure for this conversion includes:
 - New influent control structures at the GCSD site
 - A new drain pump for the FWD ponds
 - A new flow control structure at the FWD site to allow flexible flow direction within the facility (e.g. directing flow to the ponds or pump station, or combining flows)
 - Additional internal pipelines within the facilities
- A connection/capacity fee of \$135.4 million will be paid to Windsor, and this capacity cost split between RRCSD, FWD/GCSD and the unsewered communities in proportion to their ADWF values.

Site layouts for Alternative 2a will mirror those already presented in Chapter 4. The infrastructure required to convert the existing ponds at the RRCSD and GCSD WWTPs into EQ ponds is similar to Alternative 1c and is illustrated on Figures 4-4 and 4-5, respectively, as noted with Alternative 1c. The proposed infrastructure improvements to convert the existing treatment ponds at the FWD WWTP to EQ ponds are the same as those presented in Chapter 4 (Figure 4-8).

8.4.4 Conveyance Infrastructure

Relevant conveyance infrastructure includes collection system conveyance for the unsewered parcels, raw wastewater conveyance to the Windsor WWTP and potential infrastructure for the return of recycled water flows from the Windsor system.

¹⁶ These flows and loads reflect contributions from 5,319 ESDs from West County and 3,128 ESDs from the unsewered communities, totaling 8,447 ESDs. The estimate of unsewered ESDs is based on an approach used for the Monte Rio feasibility study.

¹⁷ It is assumed that flows could be equalized to 3.5 mgd. Unsewered flows would not be equalized.

¹⁸ It is assumed the GCSD and FWD flows could be equalized to the maximum 30-day average values (0.9 mgd combined total). Unsewered flows would not be equalized.

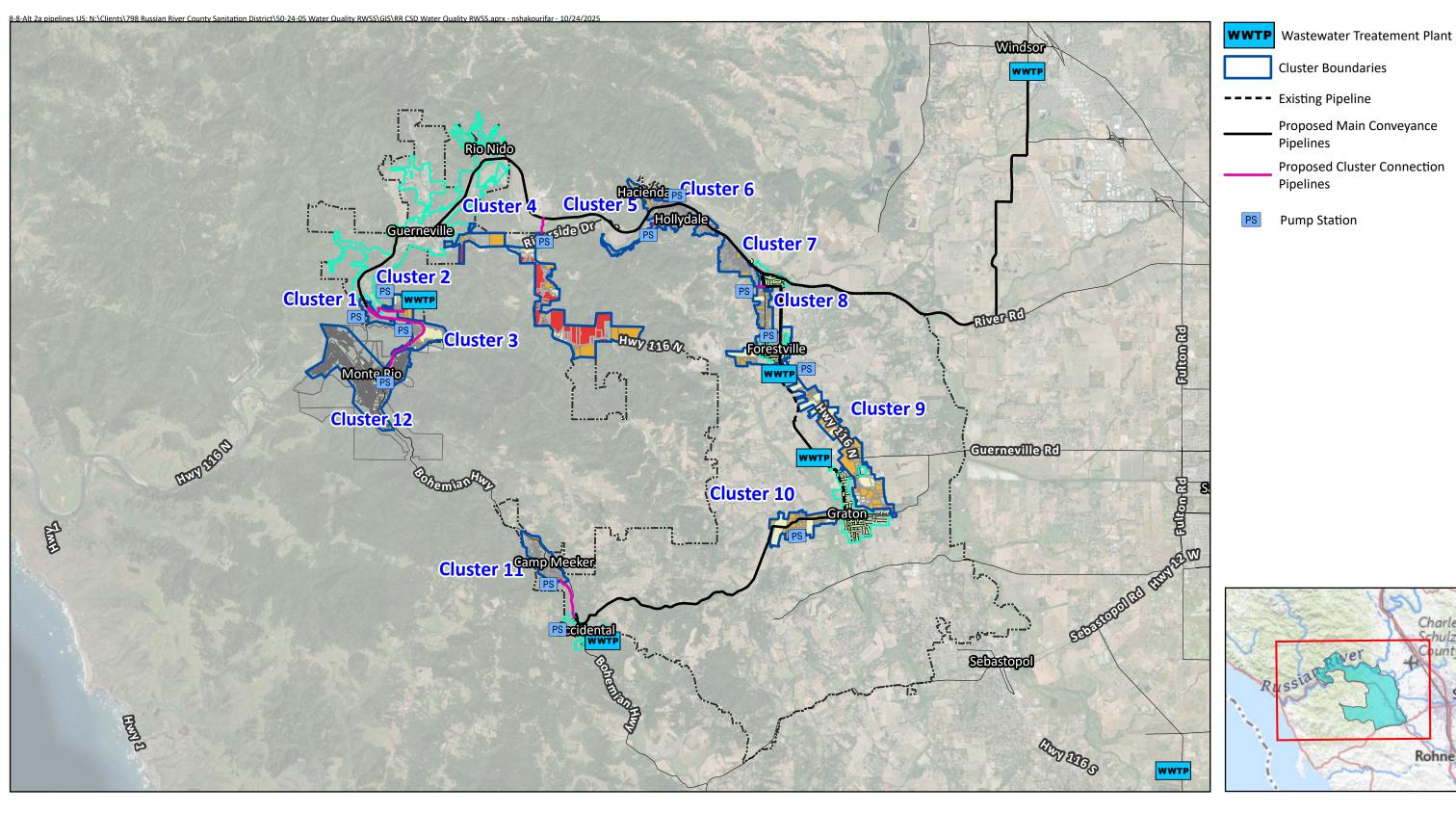


8.4.4.1 Collection System Infrastructure for Unsewered Community Clusters

A conceptual alignment is shown on Figure 8-8 of the new cluster connection pipelines and associated conveyance infrastructure to the associated WWTPs under Alternative 2a. The main force main alignment would be the same as without the unsewered flows (Figure 3-7). In addition, each cluster would have a new pipeline connection as summarized in Table 8-28.

Table 8-28. Sizing of Conveyance Infrastructure from Unsewered Areas for Alternative 2a						
Cluster(s) ^(a) Pipeline Distance, miles Pipeline Diameter, inches						
2, 4, 5, 7, 8, 9 & 11	1.6	4				
12	1.2	6				
3	0.8	8				
Total Pipeline Length	3.6					
(a) Clusters 1, 6 and 8 connect directly to the RRCSD to FWD pipeline, and Cluster 10 connects directly to the OCSD to GCSD pipeline.						





Prepared by:

Prepared for:

Russian River County Sanitation District Sonoma Water Water Quality and Recycled Water Supply Feasibility Study

Proposed Pipeline Alignments Alternative 2a

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Figure 8-8

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8.4.4.2 Raw Wastewater Conveyance to Windsor WWTP

The following major conveyances will be required for discharge or transfer of effluent and recycled water:

- New 4.3 mgd, 0.9 mgd and 1.4 mgd discharge pump stations would be constructed at the RRCSD, GCSD and FWD facilities, respectively, for raw wastewater export.
- 18.9 miles of conveyance pipeline for raw wastewater between the RRCSD and FWD/GCSD sites to a junction point and then to the Windsor WWTP. The cost of this conveyance is shared between RRCSD and FWD/GCSD facilities, based on the proportion of flow each contributes to the combined conveyance system beyond the junction.
- Rehabilitation of the existing 8-inch ductile iron pipeline between the GCSD and FWD WWTPs, and a new 0.8-mile, 6-inch extension of the existing HDPE pipeline.

Without unsewered flows, this alternative included 18.9 miles of 20- to 24-inch conveyance pipeline. The total conveyance pipeline length would remain as 18.9 miles, with the following components:

- 1.0 mile of 16-inch diameter
- 9.9 miles of 24-inch diameter
- 8.0 miles of 30-inch diameter

8.4.4.3 Potential Recycled Water Infrastructure

Chapter 6 introduces the facilities required to return exported water back to the FWD/GSCD recycled water system for local use. To support the return of recycled water, the following additional infrastructure was identified:

- A 3.0 mgd pump station located near the existing outfall in Mark West Creek, to accommodate estimated peak day recycled water flow.
- A 3.7-mile, 12-inch diameter pipeline that extends southward down Trenton-Healdsburg Road from the existing outfall location, then turns westward along River Road to Trenton Road, 19 then along Trenton Road to Covey Road, and then southward along Covey Road/Forestville Street, terminating at the FWD WWTP recycled water storage pond.

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¹⁹ The new Windsor outfall to Russian River could be accommodated by extending a pipeline from this location along River Road to a location northwest of Forestville and downstream of Sonoma Water's Mirabel water supply intake. The extension and outfall are not included in this analysis. If a regional project were to be implemented with Windsor, construction of a new outfall and the associated recycled water extension should be evaluated. Windsor staff have noted that if an outfall were to be constructed as part of the project, these costs could potentially offset some of the connection fees that have been identified by the Town.

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In addition, 185 acre-feet of additional recycled water storage would be needed. A water balance has not been developed specific to this alternative, but it is assumed recycled water usage would be similar to that under Alternative 1c. The water balance for Alternative 1c indicates no additional users (land application area) would be needed, but 185 AF of additional recycled water storage and extension of the existing system to the quarries near Forestville would be required.

Because of the uncertainty around the return of recycled water, the capital cost for Alternative 2a discussed below is presented both with and without the additional infrastructure to support return of recycled water.

8.4.5 Project Costs

The OPCC and OPTCC for Alternative 2a project elements are presented in Table 8-29, which also shows the estimated cost shares for RRCSD, FWD/GSCD and for unsewered communities. Additional details regarding these costs are provided in a Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendix G-3. The OPCC and OPTCC accounting for the potential infrastructure needed to return recycled water to the GCS/FWD system are presented in Table 8-30.

For both sets of capital costs, the following cost share approach was used:

- Costs of improvements needed to accommodate treatment of all anticipated flows and loads are split between existing users and new customers based on their respective portion of capacity provided.
- Cost of EQ improvements are applied to each community (i.e. RRCSD, GCSD and FWD).
- Any improvements or costs associated with collection systems or raw wastewater conveyance (including existing systems) are only applied to users of those systems (proportional to peak flow).





Table 8-29. OPCC and OPTCC for Alternative 2a

Facility		Cost, \$ million			
Component	Components	RRCSD	FWD/GCSD	Unsewered	Total ^(a)
Treatment		\$0.3	\$4.4	\$0.1	\$4.8
RRCSD	Convert storage ponds to EQ facility	0.3			0.3
FWD	Convert treatment ponds to EQ ponds		3.4		3.4
CCCD	Headworks improvements		0.8	0.1	0.9
GCSD	Convert treatment ponds to EQ facility		0.2		0.2
Conveyance		\$122.1	\$22.1	\$53.5	\$197.7
	0.9 mgd pump station at GCSD for secondary effluent transfer to FWD		1.7	0.7	2.4
New Pump Stations	1.4 mgd pump station at FWD for wastewater transfer to Windsor		1.8	1.0	2.8
	4.3 mgd pump station at RRCSD for wastewater transfer to Windsor	3.1		0.7	3.8
	18.9-mile of 10 to 30-inch conveyance pipeline	119.0	15.7	45.7	180.4
Pipeline	 Pipelines connections between FWD/GCSD: 0.8 mile, 6-inch pipeline extension of existing HDPE pipe Rehab existing 1.7 mile, 8-inch pipeline between GCSD and FWD 	-	2.9	1.0	3.9
	3.6 miles of 4 to 8 inch community conveyance lines	1	-	4.4	4.4
Unsewered Service Dedicated Pump S	e Area Systems (Collection Systems and Stations)	-		\$331.7	\$331.7
	Engineer's Preliminary OPCC	\$122	\$27	\$385	\$534
Engineering Design, Environmental Planning and Studies, Permitting, Construction Management, Engineering Services during Construction and Legal and Admin Costs, 25 percent of OPCC (applied to all)		31	7	96	134
Connection Fee		44.5	25.1	65.8	135.4
	Engineer's Preliminary OPTCC	\$198	\$59	\$547	\$804



Table 8-30. OPCC and OPTCC for Alternative 2a with Recycled Water Return					
		Cost, \$ million			
Facility Component	Components	RRCSD	FWD/GSCD	Unsewered	Total
Engineer's Preliminary OPTCC without Recycled Water Return		\$198	\$59	\$547	\$804
2 1 1	5.1 miles of 12-inch diameter pipeline, including conveyance to quarries	4	3	5	12
Recycled Water Infrastructure	3.0 mgd pump station from Windsor outfall	1	1	1	3
Recycled Water Storage (185 acre-feet) ^(a)		0-5 ^(a)	0-3	0-7	0-15
Total Additional OPTCC ^(a)			\$4-7	\$6-13	\$15-30
Additional Soft Costs = 25 percent of OPCC ^(a)		1-3	1-2	2-3	4-8
	Engineer's Preliminary OPTCC ^(a) \$204-211 \$64-68 \$555-563 \$823-842				\$823-842
(a) Cost ranges shown with and without recycled water storage.					

8.4.6 Operations and Maintenance Costs

This section provides a summary of the additional O&M costs associated with this alternative, focused on the following elements:

- Power costs
- Labor costs
- Chemical costs
- Equipment repair and replacement costs
- Windsor WWTP O&M rate costs

Following this summary of the additional O&M elements, the total additional O&M costs are presented along with the 20-year, present-worth lifecycle O&M cost. Additional details regarding these O&M costs are provided in the Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendices H-5 and H-6.

8.4.6.1 Power Costs

The annual power costs for Alternative 2a are summarized in Table 8-31. These costs account for increased energy demands for long-distance pumping and projected savings resulting from operational changes at the West County facilities.

Table 8-31. Annual Power Costs for Alternative 2a		
Cost Element	Cost, dollars	
Conveyance Pumps 128,500		
RRCSD, GCSD and FWD WWTPs Operation ^(a) -614,800		
Total -\$486,300		
(a) Assuming 95 percent of the current treatment power costs at RRCSD and 90 percent at FWD/GCSD would be eliminated.		



8.4.6.2 Labor Costs

Under this alternative, similar to Alternative 1c, the following labor reductions are assumed:

- 75 percent reduction at RRCSD WWTP
- 90 percent reduction in operations labor and 50 percent reduction in administrative costs at the GCSD WWTP
- 80 percent reduction at the FWD WWTP

Based on these assumptions, the estimated annual labor cost savings is \$3.6 million. This is unchanged from Chapter 4.

8.4.6.3 Chemical Costs

As shown in Table 8-32, since treatment processes are eliminated at the facilities, all existing chemical costs would be entirely saved for Alternative 2a. This is the same assumption used in Chapter 4.

Table 8-32. Annual Chemical Costs for Alternative 2a		
Cost Element Cost, dollars		
RRCSD, GCSD, and FWD WWTPs Operation ^(a) - 154,700		
(a) Assuming elimination of 100 percent of chemical costs at RRCSD, GCSD and FWD WWTPs		

8.4.6.4 Equipment Repair and Replacement Costs

A summary of the estimated major equipment repair and replacement costs for Alternative 2a is provided in Table 8-33. Replacement costs for equipment with replacement frequencies of more than 20 years were excluded from this analysis (e.g. pumps). These costs are higher than those used for Alternative 2a in Chapter 4 because of additional piping and pump capital cost from which the operating costs is calculated.

Table 8-33 Fauinment	Renair and Renlacemen	t Costs for Alternative 2a
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	Cost, dollars		
Cost Element	Annual Basis	Cost Basis (if different)	Cost Basis Assumption
Piping and Valve Maintenance and Replacement Cost	315,600	-	5 percent of mechanical and piping cost
Pumps Rebuild and Major Maintenance	15,000	158,000	30 percent of pump cost, Every 10 Years
RRCSD, GCSD and FWD WWTPs Operation ^(a)	-995,600	-	Annual
Total Annual Costs	-\$665,000	-	

⁽a) Assumes a 90 percent cost reduction for parts replacement, permitting, and testing/analysis at the RRCSD WWTP, a 50 percent reduction in SCADA-related costs at RRCSD, 80 percent reduction in equipment maintenance costs at the existing GCSD WWTP, and 60 percent reduction in existing equipment maintenance costs at the FWD WWTP.



8.4.6.5 Windsor WWTP O&M Rate Costs

The estimated annual rates for treatment of 1.1 mgd ADWF at the Windsor WWTP is approximately \$7.9 million, based on the rate currently paid for Sonoma Water's agreement for discharges from the Airport/Larkfield/Wikiup Sanitation Zone. These rates are expected to increase significantly over the next several years, likely to support the construction of new treatment facilities. However, the current analysis accounts for new facility capital costs within the connection fee.

8.4.6.6 Total Additional Annual O&M Costs

Total additional annual O&M costs are provided in Table 8-34 based on the information presented in the previous sections. Also shown are estimated cost shares, using the following approach:

- For treatment and recycled water costs, costs are divided between RRCSD, FWD/GSCD and unsewered users based on the proportional flows and loads.
- Routine O&M for conveyance refers to the major conveyance pipelines, and those costs are divided proportional to peak flows.
- For collection system costs, any new O&M is for the unsewered communities, so those are shown assigned solely to the unsewered communities.

Table 8-34. Additional Annual O&M Costs for Alternative 2a				
Service Area	RRCSD, \$	FWD/GCSD, \$	Unsewered, \$	Total Annual OpEx, \$
Treatment/Recycled	Water			
Power	-317,000	-171,000		-488,000
Labor	- 2,964,000	-598,000		-3,562,000
Chemicals	-35,000	-120,000		-155,000
Major Parts Replacement	4,800	5,100		9,800 ^(a)
Routine O&M for Treatment ^(b)	-846,000	-132,000		-978,000 ^(a)
Windsor O&M Costs	2,700,000	1,500,000	3,600,000	7,800,000
Total Treatment Annual OpEx	- \$1,457,200	\$484,100	\$3,600,000	\$2,626,800
Collection				
Routine O&M for Conveyance	186,000	33,000	79,000	298,000 ^(a)
Collection System O&M ^(c)	-	-	2,679,000	2,679,000
Total Collection Annual OpEx	\$186,000	\$33,000	\$2,758,000	\$2,977,000

⁽a) These cost items comprise the total annual equipment repair and replacement cost in Table 8-33.

⁽b) Routine O&M for treatment accounts for both paying proportional Windsor rates and proportional savings from reduced routine O&M at the West County facilities.

⁽c) Based on estimated costs shown in Table 8-5



8.4.6.7 Total 20-Year Present Worth of O&M Costs

The total 20-year, present-worth O&M costs for Alternative 2a are shown in Table 8-35. Additional details are provided in Appendices H-5 and H-6.

Table 8-35. Present Worth O&M Cost for Alternative 2a		
O&M Cost Component	Total 20-Year Cost, \$ million	
Power	-10.3	
Labor	-74.8	
Chemicals	-3.3	
Equipment Repair and Replacement	-14.1	
Windsor O&M Rates	163.8	
Unsewered Collection Systems	56.3	
Total 20-Year, Present-Worth O&M Costs	\$118	

8.4.7 Total Lifecycle Costs

A total lifecycle cost for Alternative 2a is calculated as shown in Table 8-36 using the OPTCC from Table 8-29 and total 20-year O&M costs from Table 8-35.

Table 8-36. Lifecycle Cost for Alternative 2a			
Cost Component	Cost, \$ million		
Total Project Capital Cost (CapEx) 804-842 ^(a)			
Total Present Worth O&M Costs (OpEx) 118 ^(b)			
Total Lifecycle Cost \$922-960 ^(a)			
(a) Range shown based on CapEx with and without recycled water return.(b) Additional OpEx for recycled water facilities not accounted for.			

8.4.8 Potential Annual Operating Costs per ESD

As with the other alternatives, the additional operating costs (Table 8-34) can be combined with current operating costs to review total operating costs per ESD. The same methodology used for Alternative 1c was applied for this alternative, as well (i.e. all unsewered communities combined). These per parcel costs include estimated operating costs only and do not include any costs related to debt that may arise from the implementation of a regional project. Therefore, the costs should not be interpreted as the annual charges paid by a sanitation district customer.



Table 8-37. Total Estimated Annual O&M Costs for Alternative 2a				
Category	RRCSD, \$	FWD/GCSD, \$	Unsewered, \$	Total Annual OpEx, \$
Current Annual O&M for Treatment	5.5 M ^(a)	1.6 M ^(a)	-	7.1 M
Additional Annual O&M for Treatment	- 1.5 M	0.5 M	3.6 M	2.6 M
Current Collection System O&M	3.3 M ^(a)	0.9 M ^(a)	-	4.2 M
Additional Collection System O&M	0.2 M	0.03 M	2.8 M	3.0 M
Total Annual OpEx \$7.5 M \$3.0 M \$6.4 M \$16.9 I			\$16.9 M	
Total Annual OpEx per ESD	\$2,100	\$1,800	\$2,000	
(a) Current annual O&M from FY 24/25 budge	ets.			

8.5 ALTERNATIVE 2B

Alternative 2b includes conveying untreated West County flows to the Laguna WWTP for treatment. This section presents a description of how the selected unsewered community clusters would be incorporated into this regional strategy, the associated basis of design for the facility improvements, a summary of the required facility improvements, site layout, project costs, O&M costs and total lifecycle costs.

8.5.1 Incorporation of Unsewered Clusters

The unsewered community clusters would be added in as follows:

- Clusters 1, 2, 3, and 12 would connect to a pump station that also conveys the RRCSD flows to the Laguna WWTP site.
- Clusters 4 through 7 would connect downstream to the same force main to the Laguna WWTP.
- Cluster 8 would be directly pumped first to the FWD WWTP site before combining with FWD flows in a pump station to the main force main conveying the flows detailed above.
- Cluster 9 would be conveyed initially to the GCSD pump station before being pumped into the force main from the GCSD pump station.
- Cluster 11 would initially be conveyed via the OCSD pump station and force main before traveling to the GCSD WWTP site with Cluster 10 and GCSD flows for conveyance to a pump station conveying OCSD, GCSD and Clusters 9, 10 and 11 flows to the Laguna WWTP.

8.5.2 Basis of Design

The flows and loads of interest for design of the new treatment facilities under Alternative 2b are the same as for Alternative 1c with the difference being ultimate conveyance to and treatment at the Laguna WWTP instead of the FWD WWTP. These flows and loads of interest were presented in Table 8-17. As with Alternative 2a, these flows and loads reflect contributions from 5,319 ESDs from West County and 3,128 ESDs from the unsewered communities, totaling 8,447 ESDs.



8.5.3 Required Facility Improvements

This section addresses the required improvements to convert provide EQ at the RRCSD, GSCD, and FWD treatment facilities along with the estimated connection fee to cover costs of expanded treatment at the Laguna WWTP. These improvements are as follows:

- The existing storage ponds and aeration basins at the RRCSD would be modified to provide equalization of RRCSD service area flows²⁰. A new influent control structure and internal pipelines would also be needed.
- The GCSD headworks will be upgraded with a new mechanical screening facility.
- The existing GCSD and FWD treatment ponds would be reconfigured so to provide EQ at each site²¹. Required infrastructure for this conversion includes:
 - New influent control structures at the GCSD site
 - A new drain pump for the FWD ponds
 - A new flow control structure at the FWD site to allow flexible flow direction within the facility (e.g. directing flow to the ponds or pump station, or combining flows).
 - Additional internal pipelines within the facilities.
- An estimated connection/capacity fee of \$117.6 million will need to be paid to buy capacity in the Laguna WWTP.²² These costs would be split between RRCSD, FWD/GCSD and the unsewered communities in proportion to their ADWF values.

Like Alternative 2a, site layouts showing the proposed infrastructure upgrades needed to convert the facilities into EQ facilities have been provided on Figures 4-4, 4-5 and 4-8.

8.5.4 Conveyance Infrastructure

Similar to Alternative 2a, relevant conveyance infrastructure includes collection system conveyance for the unsewered parcels, raw wastewater conveyance to the Santa Rosa collection system, and return of recycled water from the Santa Rosa system.

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²⁰ It is assumed that flows could be equalized to 3.5 mgd. Unsewered flows would not be equalized.

²¹ It is assumed the GCSD and FWD flows could be equalized to the maximum 30-day average values (0.9 mgd combined total). Unsewered flows would not be equalized.

²² Chapters 3 and 4 also discussed a \$5 million nominal cost to Santa Rosa as a placeholder for expansion of storage. If expansion of the Santa Rosa recycled water system is determined to be needed to support disposal of West County flows, the more specific infrastructure and costs identified in Chapter 6 would replace the \$5 million placeholder previously defined. Therefore, this placeholder has been removed from the estimated connection fee costs.



8.5.4.1 Collection System Infrastructure for Unsewered Community Clusters

A conceptual alignment is shown on Figure 8-9 of the new cluster connection pipelines and associated conveyance infrastructure to the associated WWTPs under Alternative 2b. The main force main alignment would change from that favored in Chapter 3 (Figure 3-8); specifically, between Guerneville and Forestville, the northern alignment would be favored. In addition, each cluster would have a new pipeline connection as summarized in Table 8-28.

Table 8-38. Sizing of Conveyance Infrastructure from Unsewered Areas for Alternative 2a				
Cluster(s) ^(a)	Pipeline Distance, miles	Pipeline Diameter, inches		
2, 4, 5, 9 & 11	1.1	4		
7 & 12	1.3	6		
1 & 3 0.8 8				
Total Pipeline Length 3.2				
(a) Clusters 1, 6 and 8 connect directly to the RRCSD to FWD pipeline, and Cluster 10 connects directly to the OCSD to GCSD pipeline.				

Details of the conveyance from the clusters would be the same as under Alternative 1c, already presented in Table 8-19.

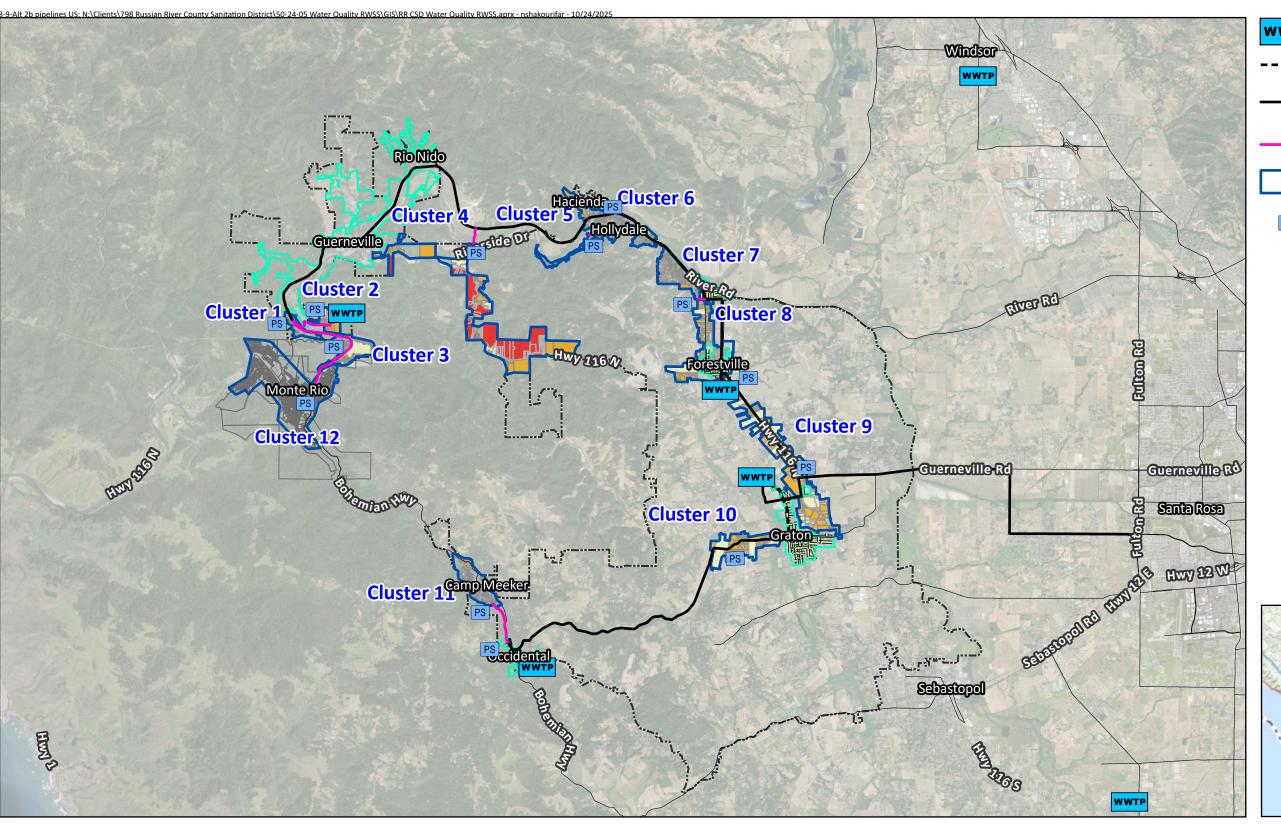
8.5.4.2 Raw Wastewater Conveyance to Santa Rosa Collection System

The following major conveyances will be required for discharge or transfer of effluent and recycled water:

- New 4.3 mgd, 0.9 mgd and 5.4 mgd discharge pump stations would be constructed at the RRCSD, GCSD and FWD facilities, respectively, to support raw wastewater export.
- 21 miles of 16- to 30-inch conveyance pipeline for raw wastewater would be constructed between the RRCSD and FWD/GCSD sites to a junction point and then to the Santa Rosa collection system near the intersection of Hall and Fulton Roads, as further discussed with conveyance infrastructure below. The cost of this conveyance is shared between RRCSD and FWD/GCSD facilities, based on the proportion of flow each contributes to the combined conveyance system beyond the junction.

Without unsewered flows, this alternative included 18.0 miles of 16- to 24-inch conveyance pipeline. The total conveyance pipeline length would increase to 20.8 miles, with the following components used for raw wastewater conveyance:

- 1.0 mile of 16-inch diameter
- 13.5 miles of 24-inch diameter
- 6.3 miles of 30-inch diameter



WWTP Wastewater Treatement Plant

---- Existing Pipeline

Proposed Main Conveyance
Pipelines

Proposed Cluster Connection
Pipelines

Cluster Boundaries

PS

Pump Station







Prepared for:

Russian River County Sanitation District
Water Quality and Recycled
Water Supply Feasibility Study



Proposed Pipeline Alignments
Alternative 2b

DRAFT

Figure 8-9



8.5.4.3 Potential Recycled Water Infrastructure

Chapter 6 also introduces the possibility of returning exported water for local recycled water use. To support the return of recycled water, the following additional infrastructure was identified:

- A 3.0 mgd pump station located near the existing recycled water storage pond adjacent to Santa Rosa Creek, to accommodate estimated peak day recycled water flow.
- A 3.6-mile, 12-inch diameter pipeline that extends westward to reach Guerneville Road, then continues westward along Guerneville Road to Highway 116, then loops down Highway 116, along Green Valley Road, and up the West County trail, terminating at the GCSD recycled storage ponds.

In addition, 185 acre-feet of additional recycled water storage would be needed. The Santa Rosa system may be able to accommodate some or all of that storage. A water balance has not been developed specific to this alternative, but it is assumed recycled water usage would be similar to that under Alternative 1c. The water balance for Alternative 1c indicates that no additional users (land application area) would be needed, but 185 AF of additional recycled water storage and expansion of the conveyance system to incorporate serve the quarries would be needed (an additional 1.4 miles of 12-inch pipeline).²³

Because of the uncertainty around the return of recycled water, the capital cost for Alternative 2b discussed below is presented both with and without the additional infrastructure to support return of recycled water.

8.5.5 Project Costs

The OPCC and OPTCC for Alternative 2b project elements are presented in Table 8-39, which also shows the estimated cost shares for RRCSD, FWD/GSCD and for unsewered communities. Additional details regarding these costs are provided in a Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendix G-4. The same cost share approach is used as for Alternative 2a. The OPCC and OPTCC accounting for return or recycled water to the FWD/GSCD system are presented in Table 8-40. Ranges are shown to allow for the possibility of that some or all of the recycled water storage could be accommodated within the Santa Rosa system.

²³ Chapters 3 and 4 included a nominal cost as a placeholder for expansion of recycled water storage under Alternative 2b. For this chapter, the costs identified in Chapter 6 would replace the \$5 million placeholder previously defined.



Table 8-39. OPCC and OPTCC for Alternative 2b

Facility		Cost, \$ million			
Component	Components	RRCSD	FWD/GCSD	Unsewered	Total ^(a)
Treatment		\$0.3	\$4.4	\$0.1	\$4.8
RRCSD	Convert storage ponds to EQ facility	0.3			0.3
FWD	Convert treatment ponds to EQ ponds		3.4		3.4
CCCD	Headworks improvements		0.8	0.1	0.9
GCSD	Convert treatment ponds to EQ facility		0.2		0.2
Conveyance		\$119.8	\$13.2	\$48.9	\$181.9
	0.9 mgd pump station at GCSD for secondary effluent transfer to FWD		1.7	0.7	2.4
New Pump Stations	5.4 mgd pump station at FWD for wastewater transfer to Santa Rosa collection system	2.6	0.2	1.3	4.1
	4.3 mgd pump station at RRCSD for wastewater transfer to Windsor	3.1		0.7	3.8
Dia alia a	21-mile of 16 to 30-inch conveyance pipeline	114.1	11.4	42.7	168.1
Pipeline	3.2 miles of 4 to 8 inch community conveyance lines			3.5	3.5
Unsewered Service Dedicated Pump	ce Area Systems (Collection Systems and Stations)			\$331.7	\$331.7
Engineer's Preliminary OPCC		\$120	\$18	\$381	\$518
Engineering Design, Environmental Planning and Studies, Permitting, Construction Management, Engineering Services during Construction and Legal and Admin Costs, 25 percent of OPCC (applied to all)		30	4	95	130
Connection Fee		47.2	26.6	48.8	122.6
Engineer's Preliminary OPTCC		\$197	\$48	\$525	\$771



Table 8-40. OPCC and OPTCC for A	Alternative 2b with Recycled Water Return
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		Cost, \$ million			
Facility Component	Components	RRCSD	FWD/GCSD	Unsewered	Total ^(a)
Engineer's Preliminary OPTCC without Recycled Water Return(a)		\$197	\$48	\$525	\$771
	5.0 miles of 12-inch diameter pipeline	8	1	3	12
Recycled Water Infrastructure	3.0 mgd pump station from Santa Rosa Reservoir	2	<1	1	3
	Recycled Water Storage (185 acre-feet) ^(b)	0-5 ^(a)	0-3	0-7	0-15
	Total Additional OPTCC(b)	\$10-15	\$1-4	\$4-11	\$15-30
Additional Soft Costs = 25 percent of OPCC ^(a)		3-4	0-1	1-3	4-8
_	Engineer's Preliminary OPTCC(b)	\$210-216	\$49-53	\$530-539	\$790-809

⁽a) OPTCC from Table 8-39 with the nominal Santa Rosa storage removed.

8.5.6 Operations and Maintenance Costs

This section provides a summary of the additional O&M costs associated with this alternative, focused on the following elements:

- Power costs
- Labor costs
- Chemical costs
- Equipment repair and replacement costs
- Laguna WWTP O&M rate costs

Following this summary of the additional O&M elements, the total additional O&M costs are presented along with the 20-year, present-worth lifecycle O&M cost. Additional details regarding these O&M costs are provided in the Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendices H-7 and H-8.

8.5.6.1 *Power Costs*

The annual power costs for Alternative 2b are summarized in Table 8-41. These costs account for increased energy demands for long-distance pumping and projected savings resulting from operational changes at the West County facilities.

Table 8-41. Annual Power Costs for Alternative 2b			
Cost Element Cost, dollars			
Conveyance Pumps 302,200			
RRCSD, GCSD and FWD WWTPs Operation ^(a) -614,800			
Total -\$312,600			
(a) Assuming 95 percent of the current treatment power costs at RRCSD and 90 percent at FWD/GCSD would be eliminated.			

⁽b) Cost ranges shown with and without recycled water storage provided by Santa Rosa.

Potential Unsewered Community Solutions



8.5.6.2 Labor Costs

The same labor cost reductions are assumed as under Alternative 2a, with the same estimated annual labor cost savings of \$3.6 million. This is unchanged from Chapter 4.

8.5.6.3 Chemical Costs

The same reduction in chemical costs is assumed as under Alternative 2a (\$154,700 a year savings), which is the same assumed in Chapter 4 for Alternatives 2a and 2b.

8.5.6.4 Equipment Repair and Replacement Costs

A summary of the estimated major equipment repair and replacement costs for Alternative 2b is provided in Table 8-42. Replacement costs for equipment with replacement frequencies of more than 20 years were excluded from this analysis (e.g. pumps). These costs are higher than those used for Alternative 2b in Chapter 4 because of additional piping and pump capital cost from which the operating costs is calculated.

Table 8-42. Equipment Repair and Replacement Costs for Alternative 2b

	Cost		
Cost Element	Annual Basis	Cost Basis (if different)	Cost Basis Assumption
Piping and Valve Maintenance and Replacement Cost	315,600	-	5 percent of mechanical and piping cost
Pumps Rebuild and Major Maintenance	12,900	135,000	30 percent of pump cost, Every 10 Years
RRCSD, GCSD and FWD WWTPs Operation ^(a)	-995,600	-	Annual
Total Annual Costs	-\$661,000		

⁽a) Assumes a 90 percent cost reduction for parts replacement, permitting, and testing/analysis at the RRCSD WWTP, a 50 percent reduction in SCADA-related costs at RRCSD, 80 percent reduction in equipment maintenance costs at the existing GCSD WWTP, and 60 percent reduction in existing equipment maintenance costs at the FWD WWTP.

8.5.6.5 Laguna WWTP O&M Rate Costs

The estimated annual rates for treatment of 1.1 mgd ADWF at Santa Rosa's Laguna WWTP are approximately \$8.8 million, based on a review of published wastewater rates and discussions with Santa Rosa staff.

8.5.6.6 Total Additional Annual O&M Costs

Total additional annual O&M costs are provided in Table 8-43 based on the information presented in the previous sections. Also shown are estimated cost shares, using the same approach as for Alternative 2a.



Table 8-43. Additional Annual O&M Costs for Alternative 2b					
Service Area	RRCSD, \$	FWD/GCSD, \$	Unsewered, \$	Total Annual OpEx, \$	
Treatment/Recycled Water					
Power	-149,000	-165,000		-314,000	
Labor	-2,964,000	-598,000		-3,562,000	
Chemicals	-35,000	-120,000		-155,000	
Major Parts Replacement	5,000	8,000		13,000 ^(a)	
Routine O&M for Treatment ^(b)	2,153,000	1,570,000		7,823,000 ^(a)	
Santa Rosa O&M Costs	3,000,000	1,700,000	4,100,000	8,800,000	
Total Treatment Annual OpEx	- \$990,000	\$695,000	\$4,100,000	\$3,805,000	
Collection					
Routine O&M for Conveyance	201,000	20,000	82,000	303,000 ^(a)	
Collection System O&M ^(c)	-	-	2,679,000	2,679,000	
Total Collection Annual OpEx	\$201,000	\$20,000	\$2,761,000	\$2,982,000	

⁽a) These cost items comprise the total annual equipment repair and replacement cost in Table 8-42.

8.5.6.7 Total 20-Year Present Worth of O&M Costs

The total 20-year, present-worth O&M costs for Alternative 2b are shown in Table 8-44. Additional details are provided in Appendices H-7 and H-8.

Table 8-44. Present Worth O&M Cost for Alternative 2b				
O&M Cost Component Total 20-Year Cost, \$ million				
Power	-6.6			
Labor	-74.8			
Chemicals	-3.3			
Equipment Repair and Replacement	-13.9			
Santa Rosa Rate	184.8			
Unsewered Collection Systems 56.3				
Total 20-Year, Present-Worth O&M Costs \$143				

⁽b) Routine O&M for treatment accounts for both paying proportional Santa Rosa rates and proportional savings from reduced routine O&M at the West County facilities.

⁽c) Based on estimated costs shown in Table 8-5.



8.5.7 Total Lifecycle Costs

A total lifecycle cost for Alternative 2b is calculated as shown in Table 8-45 using the OPTCC from Table 8-39 and total 20-year O&M costs from Table 8-44.

Table 8-45. Lifecycle Cost for Alternative 2b			
Cost Component Cost, \$ million			
Total Project Capital Cost (CapEx) 771-809 ^(a)			
Total Present Worth O&M Costs (OpEx)	143 ^(b)		
Total Lifecycle Cost \$914-952 ^(a)			
(a) Range shown based on CapEx with and without recycled water return.			
(b) Additional OpEx for recycled water facilities not accounted for.			

8.5.8 Potential Annual Operating Costs per ESD

As with the other alternatives, the additional operating costs (Table 8-43) can be combined with current operating costs to review total operating costs per ESD. The same methodology used for Alternatives 1c and 2a was applied for this alternative, as well (i.e. all unsewered communities combined). These per parcel costs include estimated operating costs only and do not include any costs related to debt that may arise from the implementation of a regional project. Therefore, the costs should not be interpreted as the annual charges paid by a sanitation district customer.

Table 8-46. Total Estimated Annual O&M Costs for Alternative 2b						
Category	RRCSD, \$	FWD/GCSD, \$	Unsewered, \$	Total Annual OpEx, \$		
Current Annual O&M for Treatment	5.5 M ^(a)	1.6 M ^(a)	-	7.1 M		
Additional Annual O&M for Treatment	- 1.0 M	0.7 M	4.1 M	3.8 M		
Current Collection System O&M	3.3 M ^(a)	0.9 M ^(a)	-	4.2 M		
Additional Collection System O&M	0.2 M	0.02 M	2.8 M	3.0 M		
Total Annual OpEx \$8.0 M \$3.2 M \$6.9 M \$18.1 M						
Total Annual OpEx per ESD \$2,200 \$1,900 \$2,200						
(a) Current annual O&M from FY 24/25 budgets.						



8.6 ALTERNATIVE 3B

Alternative 3b involves conveying all untreated wastewater flows from the RRCSD service areas to the Windsor WWTP for treatment. Flows from FWD, GCSD, and OCSD would be treated at a combined FWD/GCSD WWTP and recycled water system that is sized to accommodate zero surface water discharge (i.e. in lieu of making treatment improvements to meet the nitrogen effluent limitations that have been prescribed for surface discharge). This section presents a description of how the selected unsewered community clusters would be incorporated into this regional strategy, the associated basis of design for the facility improvements, a summary of the required facility improvements, site layout, project costs, O&M costs and total lifecycle costs.

8.6.1 Incorporation of Unsewered Clusters

Several of the unsewered community clusters would be added in with RRCSD flows as follows:

- Clusters 1, 2, 3, and 12 would connect to a pump station that also conveys the RRCSD flows to the Windsor WWTP (same as Alternative 2a).
- Clusters 4 through 7 would connect downstream to the same force main to the Windsor WWTP (same as Alternative 2a).

For Alternative 3b, flows from FWD, GCSD, and OCSD would be treated at a combined FWD/GCSD WWTP and recycled water system sized to accommodate zero surface water discharge, as with Alternative 1a. The remaining unsewered community clusters would be added in with the West County flows as follows:

- Cluster 8 would connect to the FWD WWTP (different from Alternative 1a).
- Clusters 9 through 11 would connect ultimately to the GCSD WWTP for secondary treatment (same as Alternative 1a).
- Cluster 11 would initially be conveyed via the OCSD pump station and force main before traveling to the GCSD WWTP site with Cluster 10 and GCSD flows for secondary treatment at the GCSD WWTP (same as Alternative 1a).

This section presents a description of the basis of design for the facility improvements, a summary of the required facility improvements, site layout, project costs, O&M costs and total lifecycle costs.

8.6.2 Basis of Design

The relevant design flows and loads for Alternative 3b are presented in Table 8-47 for the respective components to the Windsor and FWD WWTPs.



Table 8-47. Future West County Flows and Loads Relevant to Alternative 3b

Scenario	ADWF, mgd	Relevant Peak Flow Condition	Peak Flow, mgd	Maximum 30-Day BOD Load, lb/day
Flows and Loads to Windsor WWTP				
Projected RRCSD Flows and Loads	0.38	PDF	4.2 ^(a)	2,620
Clusters 1-7 & 12	0.37	Peak Flow	1.5	600
Total to Windsor WWTP ^(a)	0.75		5.7	3,220
Flows and Loads to FWD/GSCD WWTPs				
Projected GCSD/OCSD Flows and Loads	0.15	MWF	1.1 ^(b)	680
Projected FWD Flows and Loads	0.064	MMF	0.39 ^(b)	270
Clusters 8-11	0.13	Peak Flow	0.5	900
Total to FWD/GCSD WWTPs	0.34		2.0	1,850

⁽a) Equalized peak flow. The RRCSD Treatment Plant Master Plan indicates that Peak Day flows through the WWTP can be equalized to 5.0 mgd with use of existing Emergency Storage Pond. If 0.4 MG EQ basin is also available, flows can be equalized to 4.2 mgd. For this analysis, it is assumed flows can be equalized to 4.2 mgd.

8.6.3 Required Facility Improvements

This section addresses the required RRCSD treatment facility improvements, the required GSCD/FWD treatment facility improvements, and the required recycled water system improvements.

8.6.3.1 RRCSD Treatment Infrastructure Improvements

The RRCSD would be converted for use as EQ and export pumping only under this alternative, same as under Alternative 1c. Section 8.2.3.1 describes the components of the EQ conversion at RRCSD. The proposed infrastructure improvements for the RRCSD WWTP are unchanged from a site layout perspective from what was presented in Chapter 4 (Figure 4-4). The only distinctions are that conveyance would be to the Windsor WWTP instead of the FWD WWTP, and the wastewater pump station at the RRCSD site would be designed accordingly.

A connection/capacity fee of \$94.2 million will also need to be paid to Windsor to cover the cost of expanded treatment. These costs would be split between RRCSD and the associated unsewered communities in proportion to their respective ADWF values.

8.6.3.2 FWD/GSCD Treatment Infrastructure Improvements

The major infrastructure required for the FWD and GCSD sites under Alternative 3b is the same as for these sites under Alternative 1a, detailed in Section 8.2.3.2.

The proposed new treatment systems and infrastructure improvements for the FWD and GCSD WWTPs are to the same as those already described under Alternative 1a, with site layouts as presented on Figure 8-2 and Figure 4-3 for the respective sites, as well as Figure 8-3 for the expanded effluent storage ponds at the GCSD WWTP.

⁽b) Equalized peak flow. Chapter 4 discusses use of equalization storage to equalize peak flows to the treatment ponds.

⁽c) Total ESDs to Windsor of 5,949, comprised of 3,621 ESDs from RRCSD and 2,328 ESDs from the identified unsewered communities.



8.6.3.3 Recycled Water Infrastructure Improvements at FWD/GCSD

Water balances for the combined FWD/GCSD system have been updated from those discussed in Chapter 3 to account not the addition of unsewered flows, as well as to address the findings presented in Chapter 6 of relatively low vineyard irrigation demands and additional demands from quarries near the FWD WWTP. Updated FWD/GCSD 100-year water balance results are presented in Table 8-7 for projected West County flows with and without the unsewered clusters.

Table 8-48. FWD/GCSD Water Balance Results for Alternative 3b					
Scenario	Storage Volume Required, Irrigated/Disposal Area Reactes				
Existing Facilities					
FWD/GCSD	70 ^(a)	325 ^(b)			
FWD/GCSD Water Balance Results (1	FWD/GCSD Water Balance Results (100-Year Rainfall 75 in/yr)				
Projected	335	1,385			
Projected + Unsewered 490 2,250					
(a) Existing 7 acre-feet of storage at FWD assumed to be filled to support new treatment facilities.(b) Existing 20.5 acres of GCSD irrigation area assumed used for siting new recycled water storage.					

The updated water balance analysis indicates a need for the following:

- 1,920 acres of additional land application area²⁴.
- 420 acre-feet of new recycled water storage, corresponding to a land acquisition need of about 20 acres.

8.6.4 Conveyance Infrastructure

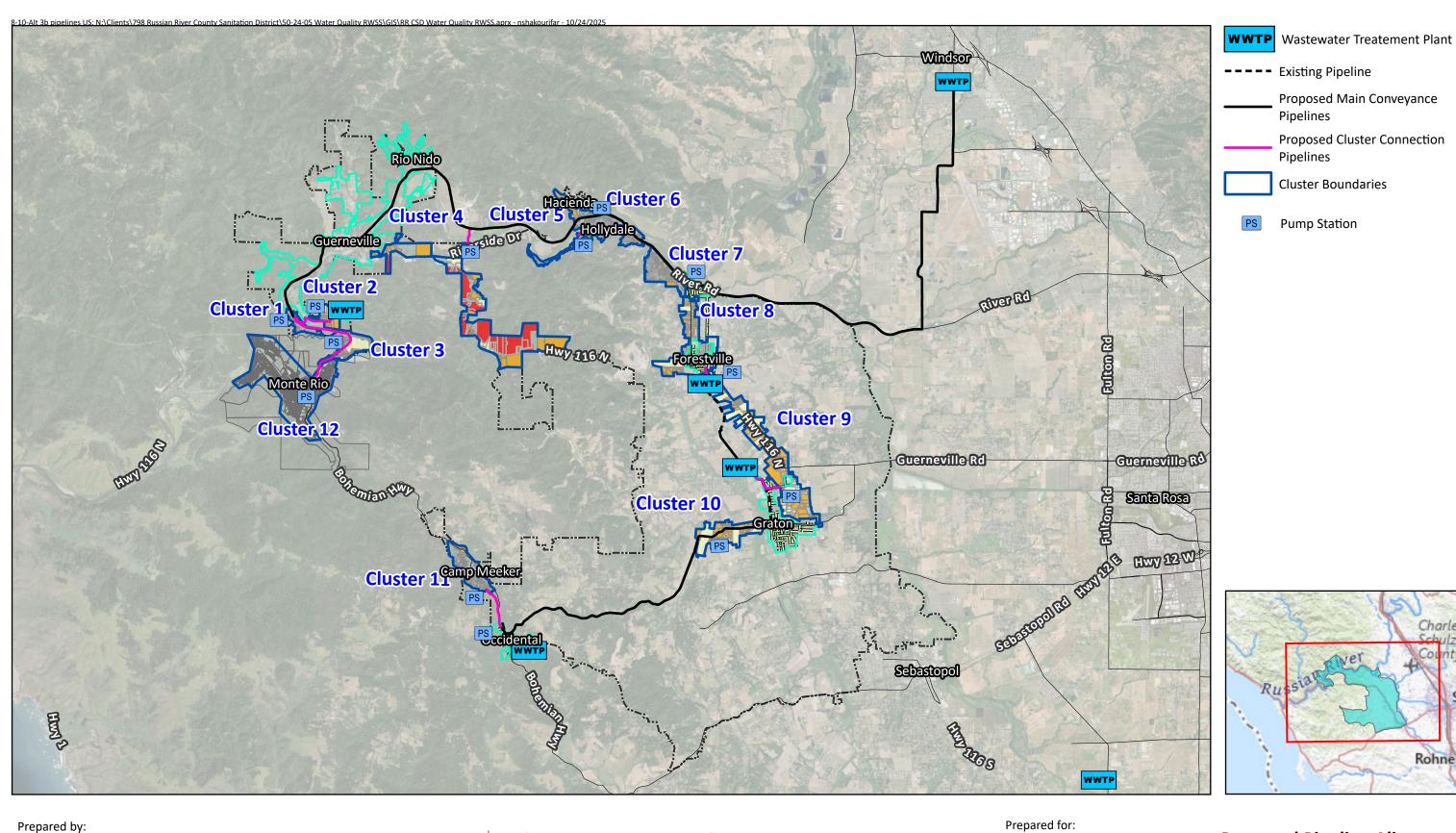
Relevant conveyance infrastructure includes collection system conveyance for the unsewered parcels, raw wastewater and secondary effluent conveyance to and from the GCSD WWTP and recycled water conveyance.

8.6.4.1 Collection System Infrastructure for Unsewered Community Clusters

A conceptual alignment is shown on Figure 8-10 of the new cluster connection pipelines and associated conveyance infrastructure to the associated WWTPs under Alternative 3b. The main force main alignment would be the same as in Chapter 3 (Figure 3-11).

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²⁴ The parallel analysis in Chapter 3 included a conclusion that the existing irrigation FWD/GCSD reuse sites would be able to accommodate the combined recycled water flow generated (without unsewered flows). These updated results reveal that even with the additional quarry demands, more than 1,000 acres of additional land application area would be needed. The addition of the quarry demands did reduce the required additional recycled water storage without unsewered flows from 310 acre-feet to 265 acre-feet.



Prepared for:

Russian River County Sanitation District Sonoma Water Water Quality and Recycled Water Supply Feasibility Study

Proposed Pipeline Alignments Alternative 3b

Existing Pipeline

Cluster Boundaries

Pump Station

Pipelines

Pipelines

Proposed Main Conveyance

Proposed Cluster Connection

DRAFT

Figure 8-10

Charles

Rohnert

Potential Unsewered Community Solutions



The distances and diameters of the conveyance pipelines are summarized in Table 8-50.

Table 8-49. Sizing of Conveyance Infrastructure from Unsewered Areas for Alternative 3b				
Cluster(s) ^(a)	Pipeline Distance, miles	Pipeline Diameter, inches		
2, 4, 5, 8, 9 & 11	2.3	4		
12	1.2	6		
3 & 8	1.0	8		
Total Pipeline Length 4.5				
(a) Clusters 6 and 7 connect directly to the RRCSD to Windsor pipeline, and Cluster 10 connects directly to the OCSD to GCSD pipeline.				

8.6.4.2 RRCSD Wastewater Conveyance to Windsor WWTP

The following major conveyances will be required for export of raw wastewater to the Windsor WWTP:

- A new 6.0 mgd discharge pump station would be constructed at the RRCSD for raw wastewater export.
- A new, 16 to 24-inch diameter, 19-mile pipeline would be constructed between the RRCSD site and Windsor WWTP.

8.6.4.3 Conveyance Between GCSD WWTP and FWD WWTP

The proposed pipeline conveyances between the GCSD and FWD WWTPs under Alternative 3b would be the same as under Alternative 1a and have already been described in Section 8.2.4.3. Two of the three pump stations would be smaller because of some additional unsewered flows being exported with RRCSD flows to the Windsor WWTP, as follows:

- Tertiary Effluent from FWD to GCSD Storage Ponds and Recycled Water Customers:
 - The existing 0.7 mgd effluent pump station at the FWD WWTP needs to be expanded to
 2.0 mgd to allow for transfer of flows from the FWD site to the ponds at the GCSD site.
- Tertiary Effluent from GCSD Storage Ponds to Recycled Water Customers:
 - A new, 3.4 mgd²⁵ recycled water pump station at the GCSD site is needed to distribute recycled water from the storage ponds to recycled water customers.

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²⁵ The recycled water pump station has been sized based on the maximum month recycled water demand from the water balance, multiplied by an assumed peak day peaking factor of 1.5 to serve recycled customers.



8.6.4.4 Recycled Water Conveyance Infrastructure

A potential expanded recycled water service area and associated recycled water main service pipelines was presented on Figure 8-5 for Alternative 1a. At a feasibility level, these conveyances are assumed to be 12-inch diameter. Alternative 3b is estimated to require only about a third of the land application area of Alternative 1a. This required acreage for Alternative 1c is assumed to include most of the northern portion of the potential land use areas shown on Figure 8-5. A total of 5 miles of conveyance pipeline is estimated, including pipelines to serve the nearby quarries. In addition, \$5,000 per acre is assumed to be needed to cover costs of additional piping and other required irrigation infrastructure on individual irrigated parcels.

8.6.5 Project Costs

The OPCC and OPTCC for Alternative 3a project elements are presented in Table 8-50, which also shows the estimated cost shares for RRCSD, FWD/GSCD and for unsewered communities. Additional details regarding these costs are provided in a Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendix G-5.

The following cost share approach was used:

- Costs of improvements needed to accommodate treatment/disposal of all anticipated flows and loads are split between existing users and new customers based on their respective portion of capacity provided.
- New FWD/GCSD customers pay a wastewater system connection fee based on existing rate structures for use of existing facilities.
- Any improvements or costs associated with collection systems or raw wastewater conveyance (including existing systems) are only applied to users of those systems (proportional to peak flow).





Table 8-50. OPCC and OPTCC for Alternative 3b

		Cost, \$ million				
Facility Component	Components	RRCSD	Unsewered to RRCSD	FWD/ GCSD	Unsewered to FWD/GCSD	Total ^(a)
Treatment		\$0.3	\$0.0	\$19.2	\$11.7	\$31.2
RRCSD	Convert storage ponds to EQ facility	0.3				0.3
FWD	New SAF/cloth disk filtration facility and disinfection expansion			12.7	7.9	20.6
	Headworks improvements			0.7	0.2	0.9
GCSD	Convert treatment ponds to complete mix system			5.8	3.6	9.4
Conveyance		\$116.7	\$53.9	\$11.7	\$8.4	\$190.7
	1.5 mgd pump station at GCSD for secondary effluent transfer to FWD			1.9	0.6	2.5
New Pump	3.4 mgd pump station at GCSD for tertiary effluent delivery to distribution system			2.3	0.7	3.0
Stations	2.0 mgd pump station at FWD for tertiary effluent transfer to GCSD			2.1	0.6	2.7
	6.0 mgd pump station at RRCSD for wastewater transfer to Windsor	3.0	1.3			4.3
	19-miles of 16 to 24-inch conveyance pipelines to Windsor	113.7	48.7			162.5
Pipeline	 Pipeline Connections Between FWD/GCSD: 0.8 mile, 6-inch pipeline extension of existing HDPE pipe 1.7 mile, 14-inch pipeline Rehab existing 1.7 mile, 8-inch ductile iron pipeline 			5.5	4.0	9.5
	4.5 miles of 4-8-inch community conveyance lines		3.8		2.4	6.2
Recycled Water				\$33.0	\$20.1	\$53.1
Land Application Area	1,920 acres			6.0	3.6	9.6
•	420 acre-feet (GCSD)			15.8	9.6	25.4
Storage	20 acres land purchase			1.2	0.8	2.0
Pipeline to Use Areas	5 miles of 6- to 12-inch conveyance pipeline			10.0	6.1	16.1
Unsewered Serv Dedicated Pump	ice Area Systems (Collection Systems and Stations)		\$237.9		\$93.8	\$331.7
	Engineer's Preliminary OPCC	\$117	\$342	\$64	\$134	\$607
Permitting, Cons	gn, Environmental Planning and Studies, truction Management, ESDC and Legal and percent of OPCC (applied to all)	29	73	16	34	152
Connection Fees		44.8	50.0			94.8
	Engineer's Preliminary OPTCC	\$191	\$415	\$80	\$168	\$854
Wastewa	ater Agency's Treatment Plant Connection Fees			- \$8	\$8	
	OPTCC with Connection Fees Included			\$72	\$175	
ESDC = engineering	services during construction	•	•		•	



8.6.6 Operations and Maintenance Costs

This section provides a summary of the additional O&M costs associated with this alternative, focused on the following elements:

- Power costs
- Labor costs
- Chemical costs
- Equipment repair and replacement costs
- Windsor O&M rate costs

Following this summary of the additional O&M elements, the total additional O&M costs are presented along with the 20-year, present-worth lifecycle O&M cost. Additional details regarding these O&M costs are provided in the Basis for Cost Estimating TM in Appendix A and detailed cost tables in Appendices H-9 and H-10.

8.6.6.1 Power Costs

The annual power costs for Alternative 3b are summarized in Table 8-51. These costs account for the energy demands of new treatment systems and conveyance infrastructure, as well as projected savings resulting from operational changes at the RRCSD and GCSD WWTPs. A net savings would be realized, but much less than under the export only alternatives (\$300,000 to \$500,000 a year savings).

Table 8-51. Annual Power Costs for Alternative 3b				
Cost Element Annual Cost, dollars				
Treatment Pond at GCSD WWTP	173,200			
RRCSD, GCSD and FWD WWTPs Operation ^(a) -565,800				
Cloth Disk Filtration 900				
SAF 12,800				
Conveyance Pumps 168,200				
Total -\$210,700				
(a) Assuming elimination of 90, 95 and 60 percent of GCSD, RRCSD and FWD current treatment power costs.				

8.6.6.2 Labor Costs

As with Alternative 1a, which includes the same approach for the FWD/GCSD systems, two additional full-time employees are assumed to be required for FWD/GCSD WWTP recycled water system O&M. This equates to a total labor cost of \$400,000 per year for these systems.

For the RRCSD component, a 75 percent reduction in labor costs at the RRCSD WWTP is assumed, same as in Chapter 4 for this alternative. This assumption results in an estimated annual savings of \$3.0 million.



8.6.6.3 Chemical Costs

The chemical costs for the new treatment systems at FWD, as well as the savings at GCSD, are similar to those presented for Alternative 1a in Section 0. Additionally, further chemical cost savings will occur at RRCSD due to the elimination of treatment at that facility. A summary of estimated annual chemical cost Is provided in Table 8-52. The savings assumed at the RRCSD and GCSD WWTP are the same as those used in Chapter 4. Additional SAF chemical and chlorine costs would be incurred to treat the unsewered flows, proportional to the increased flows.

Table 8-52. Annual Chemical Costs for Alternative 3b							
Cost Element	Annual Cost, dollars						
SAF	89,600 ^(a)						
ССВ	32,000 ^(b)						
RRCSD and GCSD WWTPs Operation	-104,700 ^(c)						
Total	\$16,900						

⁽a) Based on chemical usage at the existing GCSD SAF facility.

8.6.6.4 Equipment Repair and Replacement Costs

A summary of the estimated major equipment repair and replacement costs for Alternative 3b is provided in Table 8-53. Replacement costs for equipment with replacement frequencies of more than 20 years were excluded from this analysis (e.g. pumps). These costs are higher than those used for Alternative 3b in Chapter 4 because of capital cost from which the various operating costs components are calculated.

⁽b) Additional chlorine gas and sodium bisulfite required for the expanded CCB at the FWD WWTP are estimated using FWD current design criteria and the projected increase in annual average flow from GCSD.

⁽c) Assuming elimination of 100 percent of GCSD and RRCSD chemical costs.



Table 8-53. Equipment Repair and Replacement Costs for Alternative 3b

	Cost		
Cost Element	Annual Basis	Cost Basis (if different)	Cost Basis Assumption
Filter Cloth Replacement	2,700	-	Annual
Routine O&M ^(a)	9,400	-	Annual
Piping and Valve Maintenance and Replacement Cost	344,900	-	5 percent of mechanical and piping cost
Instrumentation Maintenance	3,800	80,000	5 percent of instrumentation and controls cost, Year 15
Pumps Rebuild and Major Maintenance	5,300	56,000	30 percent of pump cost, Every 10 Years
10-Year Equipment Replacement ^(b)	200	2,100	Every 10 years
Major Equipment Replacement	2,100	43,200	Every 15 Years
RRCSD WWTP Operation ^(c)	-853,600	-	Annual
Total Annual Costs	-\$485,200	- ^	

⁽a) Includes cloth disk filtration routine lubrication of backwash pumps, drive motor and gear box, SAF parts replacement, pond cleaning, pond blower filter/belt/ oil changes.

8.6.6.5 Windsor WWTP O&M Rate Costs

The annual rates for 0.77 mgd from the RRCSD WWTP to the Windsor WWTP are estimated to be \$5.4 million, based on the rate currently paid for discharges from the Airport area, with an adjustment to reflect the smaller flow for RRCSD. As noted with discussion of these rates with Alternative 2a, the Airport rates are expected to increase significantly in the coming years, likely to support the construction of new treatment facilities, but the current analysis accounts for new facility capital costs within the connection fees.

8.6.6.1 Total Additional Annual O&M Costs

Total additional annual O&M costs are provided in Table 8-54 based on the information presented in the previous sections. Also shown are estimated cost shares, using the same approach as Alternatives 2a and 1a for RRCSD and FWD/GSCD, respectively.

⁽b) Includes cloth disk filtration main "V-Ring" seal replacement.

⁽c) Assumes a 90 percent cost reduction for parts replacement, permitting, and testing/analysis at the RRCSD WWTP and a 50 percent reduction in SCADA-related costs at RRCSD.



Table 8-54. Additional Annual O&M Costs for Alternative 3b							
Service Area	RRCSD, \$	Windsor Unsewered, \$	FWD/0	GCSD, \$	Total Annual OpEx, \$		
Treatment/Recycled V	Vater						
Power	-287,000			75,000	-212,000		
Labor	- 2,964,00			400,000	-2,564,000		
Chemicals	-35,000			52,000	17,000		
Major Parts Replacement	2,000		8,000 1		12,000 ^(a)		
Routine O&M for Treatment	-853,000 ^(b)			83,000	-770,000 ^(a)		
Windsor O&M Costs	2,700,000	2,700,000			5,400,000		
Total Treatment Annual OpEx	-\$1,437,000	\$2,700,000		\$620,000	\$1,883,000		
Collection	RRCSD	Unsewered	FWD/GSCD	Unsewered	Total		
Routine O&M for Conveyance	185,000	67,000	22,000	3,000	277,000 ^(a)		
Collection System O&M ^(c)	-	2,322,000	-	357,000	2,679,000		
Total Collection Annual OpEx	\$187,000	\$2,387,000	\$22,000	\$360,000	\$2,956,000		

⁽a) These cost items comprise the total annual equipment repair and replacement cost in Table 8-53.

8.6.6.2 Total 20-Year Present Worth of O&M Costs

The total 20-year, present-worth O&M costs for Alternative 3b are shown in Table 8-55. Additional details are provided in Appendices H-9 and H-10.

Table 8-55. Present Worth O&M Cost for Alternative 3b							
O&M Cost Component	Total 20-Year Cost, \$ million						
Power	-4.5						
Labor	-53.8						
Chemicals	0.4						
Equipment Repair and Replacement	-10.2						
Windsor Rate	113.4						
Unsewered Collection Systems	56.3						
Total 20-Year, Present-Worth O&M Costs	\$102						

⁽b) Accounts for proportional savings from reduced routine O&M at the RRCSD WWTP.

⁽c) Based on estimated costs shown in Table 8 5.



8.6.7 Total Lifecycle Costs

A total lifecycle cost for Alternative 3b is calculated as shown in Table 8-56 using the OPTCC from Table 8-50 and total 20-year O&M costs from Table 8-55.

Table 8-56. Lifecycle Cost for Alternative 3b								
Cost Component	Cost, \$ million							
Total Project Capital Cost (CapEx)	854							
Total Present Worth O&M Costs (OpEx)	102							
Total	\$956							

8.6.8 Potential Annual Operating Costs per ESD

As with the other alternatives, the additional operating costs (Table 8-54) can be combined with current operating costs to review total operating costs per ESD. The same methodology used for the respective alternatives (Alternatives 2a and 1a) was applied for this alternative, as well. These per parcel costs include estimated operating costs only and do not include any costs related to debt that may arise from the implementation of a regional project. Therefore, the costs should not be interpreted as the annual charges paid by a sanitation district customer.

Table 8-57. Total Estimated Annual O&M Costs for Alternative 3b								
Category	RRCSD, \$	Windsor Unsewered, \$	FWD/GCSD, \$	FWD/GSCD Unsewered, \$	Total Annual OpEx, \$			
Current Annual O&M for Treatment	5.5 M ^(a)	-	1.1 M ^(a)	0.5 M	7.1 M			
Additional Annual O&M for Treatment	- 1.5 M	2.7 M	0.4 M	0.2 M	1.9 M			
Current Collection System O&M	3.3 M ^(a)	-	0.9 M ^(a)	-	4.2 M			
Additional Collection System O&M	0.2 M	2.4 M	0.02 M	0.4 M	3.0 M			
Total Annual OpEx	\$7.5 M	\$5.1 M	\$2.4 M	\$1.1 M	\$16.2 M			
Total Annual OpEx per ESD	\$2,100	\$2,100	\$1,400	\$1,400				
(a) Current annual O&M from FY 24/25 budgets.								



8.7 COMPARISON OF ALTERNATIVES

This section provides a summary and comparison of the alternatives, focused on capital cost per ESD for the unsewered communities, total lifecycle costs and updated screening criteria scoring to inform drawing conclusions with regards to a best alternative considering the opportunities to bring in flows from the unsewered communities.

8.7.1 Summary of Estimated Lifecycle Costs

The estimated project lifecycle costs for all five preferred alternatives are summarized in Table 8-58, which shows capital costs, 20-year, present-worth O&M costs, and total 20-year lifecycle costs. The capital cost share between RRCSD and FWD/GCSD is also shown.

Table 8-58. Summary of Estimated Lifecycle Costs, \$ million								
Estimated Cost Component	Alternative 1a	Alternative 1c	Alternative 2a	Alternative 2b	Alternative 3b			
Capital Cost	678	763	804	771	854			
RRCSD Share	17	192	198	197	191			
FWD/GSCD Share	75	52	59	48	72			
Unsewered Share	588	519	547	525	590			
20-Year, Present-Worth O&M (OpEx)	80	-4	118	143	102			
Estimated Total 20-Year Lifecyle Cost	758	759	922	914	956			
Estimated Total 20-Year Lifecycle Cost with Returning Recycled Water to West County	not applicable (n/a)	n/a	960	952	n/a			

The project costs presented in Table 8-58 shows Alternatives 1a and 1c as the least cost alternatives with similar total lifecycle costs. While Alternative 1c has a much lower (and the lowest) operating costs, the significantly lower capital cost of Alternative 1a compensates.

In contrast, Alternative 3b represents the highest-cost scenario, driven by (1) the highest capital cost and (2) relatively high operating costs. Alternatives 2a and 2b have slightly lower total costs than Alternative 3b, but are still much higher than Alternatives 1a and 1c, driven by (1) relatively high capital cost and (2) the highest operating costs values.



8.7.2 Potential Costs for Unsewered Community Clusters

This section addresses the alternatives with respect to the potential costs to the unsewered communities.

8.7.2.1 Potential Capital Costs

Reviewing the capital cost per ESD for each of the community clusters can inform whether one or more alternatives would represent significant unit cost savings over other alternatives. Estimated capital cost per ESD for the unsewered communities has been calculated as follows:

- Number of ESDs for each cluster is calculated from the total number of parcels in Table 8-4, divided by the single-family residential unit flow of 158 gallons per day established in the Monte Rio Study Report.
- The share of unsewered OPCC from the respective alternatives is taken from the project cost tables (Table 8-9, Table 8-20, Table 8-29, Table 8-39 and Table 8-50).
- These OPCC shares are split out to the respective clusters as follows:
 - proportional to peak flows for pump station costs
 - proportional to average flows for all other components except pipelines
 - for pipelines, total costs for relevant segments to each cluster have been used (For example, under Alternative 1c, clusters near the RRCSD WWTP have to travel through longer length of pipelines than for those closer to the FWD WWTP.)
- The components costs are summed to give a total OPCC for each cluster.
- A factor of 25 percent is added for soft cost, and the OPCC and soft costs added to give an OPTCC for each cluster.
- The cluster OPTCC is divided by the respective number of ESDs to give the capital cost per ESD.

The ESD and capital cost per ESD values are presented in Table 8-59, which also shows an ESD-weighted average capital cost per ESD for comparison across alternatives. These unit capital costs should be based on feasibility-level costs to provide a general (order of magnitude) average cost across the community. Individual parcel costs within each community could vary significantly (±50 percent) depending on the complexity of serving the parcel.



Table 8-59. Estimated Capital Cost per ESD for Unsewered Communities, \$ million

ID	Name	Number of ESDs	Alternative 1a	Alternative 1c	Alternative 2a	Alternative 2b	Alternative 3b
1	Guerneville South of River	55	0.16	0.18	0.16	0.16	0.18
2	Guerneville North of River	58	0.16	0.19	0.16	0.16	0.19
3	Northwood	196	0.14	0.16	0.14	0.14	0.16
4	Hwy 116 East of Guerneville	119	0.20	0.19	0.14	0.14	0.16
5	Summerhome Park Road	103	0.18	0.17	0.14	0.14	0.16
6	Hacienda and Hollydale	443	0.16	0.15	0.12	0.12	0.14
7	River Road North of Forestville	454	0.15	0.14	0.12	0.12	0.14
8	Forestville	173	0.15	0.14	0.13	0.12	0.16
9	Hwy 116 between Forestville and Graton	161	0.15	0.13	0.12	0.11	0.15
10	Graton West	70	0.17	0.16	0.15	0.14	0.18
11	Camp Meeker	376	0.15	0.14	0.13	0.12	0.16
12	Monte Rio/Villa Grande	943	0.12	0.14	0.12	0.12	0.14
	Total ESDs	3,151					
	ESD-Weighted Average Cap	x per ESD	0.14	0.15	0.13 ^(a)	0.12 ^(b)	0.15

⁽a) Under Alternative 2a, weighted average is the same with or without accounting for recycled water return.

The following observations are made from the capital cost per ESD results for the unsewered communities in Table 8-59:

- Capital cost per ESD varies from \$110,000 to \$200,000, with variation within each cluster of \$20,000 to \$60,000.
- No one alternative represents a global minimum or maximum for all clusters, reflected by the fact that the weighted-average values are all similar, between \$120,000 and \$150,000 per ESD.
- The highest capital cost per ESD values are generally for Clusters 1 through 4 near the RRCSD WWTP, while the lowest capital cost per ESD values are generally for Clusters 7 through 9 closer to the FWD WWTP. These results reflect higher costs associated with pipeline conveyance. Cluster 4, in particular, is relatively far from the existing WWTPs.

⁽b) Under Alternative 2b, weighted average is slightly higher when accounting for recycled water return (0.13 with vs. 0.12 without).



8.7.2.2 Potential Annual Operating Costs per ESD

The potential annual rates per ESD for the different alternatives shown in previous tables are summarized in Table 8-60, from which the following observations are made:

- Alternative 1c would have the lowest rates for all communities. This is primarily driven by the fact that treatment-related operating costs would be consolidated at a single facility,
- For the unsewered community clusters, the local alternatives (1a and 1c) offer the lowest potential annual operating cost.

The rate costs are based on a feasibility-level analysis to provide a general (order of magnitude) average annual operating costs. These costs also include estimated operating costs only and do not include any costs related to debt that may arise from the implementation of a regional project. Therefore, the costs should not be interpreted as the annual charges paid by a sanitation district customer.

Table 8-60. Summary of Annual Rates per ESD									
Community	Alternative 1a	Alternative 1c	Alternative 2a	Alternative 2b	Alternative 3b				
RRCSD	\$2,100	\$1,600	\$2,100	\$2,200	\$2,100				
RRCSD Unsewered	\$2,100	n/a	n/a	n/a	n/a				
FWD/GSCD	\$1,200	\$800	\$1,800	\$1,900	\$1,400				
FWD/GSCD Unsewered	\$1,500	\$1,200	n/a	n/a	\$1,400				
Regional Facility Unsewered	n/a	n/a	\$2,000	\$2,200	\$2,100				

8.7.3 Updated Screening Criteria Scoring

Qualitative screening scores were provided in Chapter 3 (Table 3-12) for all eight of the original alternatives and repeated in Chapter 4 (Table 4-35) for the five preferred alternatives. The scores in Table 4-35 for the preferred alternatives have been reviewed in light of the additional information developed for the current chapter, namely impacts of adding in the unsewered communities. A summary of the screening scores for each criterion for each alternative is provided in Table 8-61. Detailed explanations of the scoring are provided in Table 8-62.



Table 8-61. Summary of Screening Criteria Scoring

Criteria	Alternative 1a	Alternative 1c	Alternative 2a	Alternative 2b	Alternative 3b
Reliability/ Ease of Operation	1.0	4.0	5.0	5.0	3.0
Long-Term Regulatory Compliance	3.5	4.0	4.0	5.0	4.5
Flexibility for Adding Critical Unsewered Communities	1.0	5.0	4.0	4.0	2.5
Local Recycled Water Benefits	5.0	2.0	1.0	1.0	4.0
Environmental	2.0	4.5	5.0	5.0	3.5
Resiliency	1.0	3.0	4.0	4.0	2.5
Ease of Implementation	2.0	4.5	3.0	2.5	1.0
Average Score	2.1	3.9	3.8	3.8	3.0
Total Weighted ^(a) Score	15.5	27.0	26.0	26.5	21.0





Table 8-61. Screening Criteria Scoring Details

	Scoring of Alternative ^(a)					
Criteria	1a: Two Local Facilities	1c: One Facility at FWD	2a: Export to Windsor	2b: Export to Santa Rosa	3b: Treat at GCSD/FWD; Export RRCSD to Windsor	Scoring Comments
Reliability/ Ease of Operation	1	4	5	5	3	 The Export Scenarios (2a and 2b) involve sending flow to larger, regional facilities and would offer greater reliability and ease of operation for West County agencies. The remaining alternatives were scored lower depending on how many, smaller facilities would continue to be operated and whether additional flow equalization would be provided. The lowest score assigned to the alternative that continues to have three facilities in operation with no additional equalization for peak flows (1a). Alternative 1c scored the highest of the local facility scenarios, as it provides for equalization at all three existing sites and provides for a new treatment system and a new discharge into the Russian River. Alternative 3b was rated between Alternative 1a and Alternative 2a. The RRCSD system would be similar under both options, but the FWD treatment system would involve two different treatment plants with no additional equalization and a large, complex recycled water system.
Long-Term Regulatory Compliance	3.5	4	4	5	4.5	 The Export Scenarios (2a and 2b) involve sending flow to larger, regional facilities and would offer greater potential for adjusting to future regulatory changes. The local facilities would be operated to provide equalization, which would also help to mitigate compliance concerns related to peak flows. Alternative 2b ranks higher because the Santa Rosa system has limited need for surface water discharge, while the Windsor facility is reliant on discharges each year – driving more stringent effluent limitations. Alternative 3b scored the second highest because it includes a GSCD/FWD facility that has zero surface water discharge. A zero-surface water discharge approach provides significant resilience to changing regulatory requirements. The two remaining alternatives include one local treatment facility that discharges to surface water. Providing all treatment at FWD (1c) scored the highest of the local treatment surface water discharge scenarios. Although all the flow would be discharged to surface waters, the new facility would be properly sized to accommodate this flow. Moreover, all three of the existing treatment systems would have equalization to help mitigate peak flow concerns. Finally, the alternative includes a new discharge location into the Russian River, providing significant dilution for discharged flows. Alternative 1a scored high on one hand because it provides zero surface water discharge for a portion of the flow – thus having a higher level of resilience than discharging to surface waters. However, it also continues to provide treatment at the RRCSD WWTP, which is an older treatment facility with ongoing peak flow issues.
Flexibility for Future Expansion	1	5	4	4	2.5	 Alternative 1c provides the highest flexibility to accommodate regional growth, as the treatment and conveyance systems could be expanded as new development occurs (or additional communities seek to eliminate OWTS). Scenarios that involve export to Santa Rosa or Windsor score lower. Although the treatment systems could be readily expanded, the capacity to accept flows could be limited by initial pipeline sizing. Scenario 1a would require expansion of an already significant recycled water system as new development is brought online – complicating the ability for expansion. Scenario 3b scores between Scenario 1a and 2a, as it provides elements of both approaches.
Local Recycled Water Benefits	5.0	2	1	1	4.0	 Scenario 1a ranks the highest because it significantly expands recycled water use in the West County area. The combination scenario (3b) also scored high because it significantly expands reuse in the Graton/Forestville area. However, it was derated because it involves export of recycled water from the Guerneville area. Scenario 1c scored lower than 3b because it would retain all existing recycled water uses in the Forestville/Graton area, but export of recycled water from the Guerneville area. The export scenarios (2a and 2b) scored the lowest because they involve loss of all locally available recycled water.
Environmental	2	4.5	5	5	3.5	 Alternatives 2a and 2b would include pumping of wastewater to large, conventional wastewater facilities that in themselves use significant energy. However, overall energy usage would be lower than providing treatment at multiple, smaller facilities. It also provides for centralized treatment that provides more efficient use of resources. Alternative 1c scored the second highest because although requires the least amount of additional pipeline construction and has the smallest overall footprint, it is slightly derated due to the need to construct a new outfall in the Russian River. Alternative 1a scored the lowest due to the need for major construction at the FWD and GCSD WWTPs and the need for construction of a substantial pipeline network to expand the recycled water use area around the Graton/Forestville communities. Alternative 3b is similar to 2a (both involve export to Windsor) but is derated for the same reasons that Alternative 1a is rated low.
Resiliency	1	3	4	4	2.5	 The export scenarios (2a and 2b) will be more resilient because the larger, regional facilities will have more resources to maintain resiliency over the long term. These scenarios therefore scored the highest. Neither option received the highest rating (5) because of a construction of the pipeline along River Road, which would be susceptible to seismic and flooding impacts. Alternatives that continue to rely on the Russian River and Graton WWTPs are scored the lowest (1a) due to their reliance on a treatment facility that is near/in a floodplain and thus particularly vulnerable to seismic and flooding impacts. Alternative 1c is considered more resilient than 1a because the treatment system would be at the FWD WWTP, which is more protected from flooding than the other two local facilities. Alternative 3b was ranked between 1a and 2a because it provides elements of both options.

Table 8-61. Screening Criteria Scoring Details

	Scoring of Alternative ^(a)					
Criteria	1a: Two Local Facilities	1c: One Facility at FWD	2a: Export to Windsor	2b: Export to Santa Rosa	3b: Treat at GCSD/FWD; Export RRCSD to Windsor	Scoring Comments
Ease of Implementation	2	4.5	3	2.5	1	 Alternative 1c would be the easiest to implement, as it includes agencies that are currently involved with this study. Although it would require expanding and/or constructing new facilities, these improvements are necessary to support ongoing operations at the FWD/GCSD WWTPs. This approach could also be phased to optimize funding opportunities and allow for growth/expansion over time. This alternative was slightly derated because they would likely require modifying the existing governance structure for the West County utilities. Alternative 1a ranked relatively low. While it does involve the least amount of treatment infrastructure, it will take significant time to develop and implement a complex recycled water system to achieve zero surface water discharge. Moreover, FWD/GCSD are operating under a timeline to complete necessary upgrades for surface water discharge and it would be challenging to develop and implement such a complex recycled water system in that timeframe. The export alternatives (2a and 2b) are large linear construction projects that would bring some construction complexity but would be fairly easy to construct. However, it would generally not be feasible to phase these projects and right sizing infrastructure could be complicated. These alternatives would also require coordination with the Town of Windsor or the City of Santa Rosa for a discharge agreement. Coordinating with the City of Santa Rosa will likely require more negotiation than with the Town of Windsor due to both the uncertainty in costs and a governance structure involving multiple external agencies. The combination scenario (3b) would also be relatively simple to construct from a treatment perspective (like 1a). However, the large export pipeline and complex recycled water system make this a more complicated approach. Like the other regional options, this alternative provides less flexibility for phasing and requires more coordination than the local treatment alternatives. There
Average Score	3.0	3.2	3.4	3.3	3.3	
Total Score	21.0	22.5	23.5	23.0	23.0	

⁽a) Not all categories include scores of 1 or 2 because the relative scoring is similar among the alternatives.



8.7.4 Comparison of Alternatives

A plot of the average (or total) screening scores against the estimated lifecycle costs from Table 8-58 is provided on Figure 8-11 to compare alternatives in terms of both costs and qualitative criteria scores.

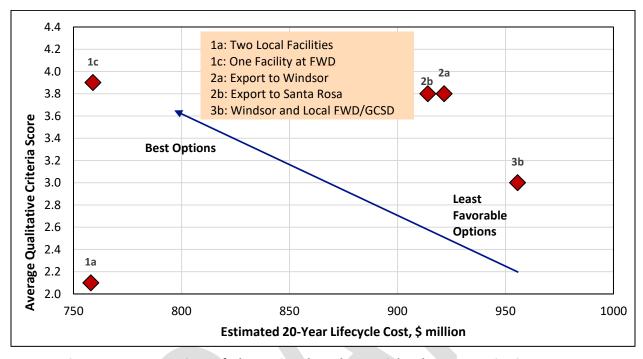


Figure 8-11. Comparison of Alternatives based on Weighted Average Criteria Score vs. 20-Year Lifecycle Cost

The results on Figure 8-11 reveal Alternative 1c as the best alternative, with the estimated lifecycle cost only slightly above Alternative 1a and highest criteria score. The alternatives with export components (2a, 2b, and 3b) continue to have significantly higher lifecycle costs. It is noted that Alternative 1a would have similar qualitative criteria score to Alternative 1c if Alternative 1a included upgraded treatment and surface water discharge. Moreover, lifecycle costs for Alternative 1a would still likely be lower than Alternative 1c if the surface water discharge approach were assumed based on the following:

- RRCSD estimated project costs under Alternative 1a is \$23 million (see Table 8-9) compared
 to \$192 million under Alternative 1c (see Table 8-20). This suggests that costs to RRCSD are
 significantly lower by maintaining treatment at the existing facility.
- FWD/GSCD estimated project costs under Alternative 1a is \$94 million (see Table 8-9) compared to \$52 million under Alternative 1c (see Table 8-20). Even with savings realized from the economies of scale under Alternative 1c, the analysis suggests that costs to construct a new treatment plant to allow for surface water discharge will be more cost effective than expanding the recycled water system to achieve zero surface water discharge.

Chapter 8

Potential Unsewered Community Solutions



Future studies completed by FWD/GCSD to asses strategies for meeting the near term treatment capacity and water quality needs for their respective facilities should consider evaluation of a modified Alternative 1a that assumes a combined FWD/GSCD facility is constructed to provide the water quality required for continued seasonal surface water discharge.

Adding recycled water return for Alternatives 2a and 2b would result in those two alternatives ranking slightly higher than Alternative 1c on the qualitative scoring. However, this approach would add lifecycle costs, making those alternatives even more expensive. Therefore, the inclusion of return of recycled water does not change the overall recommendations from this study.

